THE NATIONAL ACADEMIES PRESS

This PDF is available at http://nap.edu/23225



SCRIB? Nels-Only Document 82 (Project 4-19 (2)): Appendises & through IF of the Carboster's Final Report
Validation of Performance-
Related Test of Aggregates for
Use in Hot-Mix Asphalt
Pavements
Appendixes A through F of the Contractor's
Final Report
Prepared for National Cooperative Highway Research Program
TRANSPORTATION RESEARCH BOARD of the hattowa adapted
Submitted by:
Thomas D. White Minimippi State University Minimippi State, MS
John I. Haddeck Packar University West Lafeyette. IN
March 2006

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

DETAILS

0 pages | null | PAPERBACK ISBN 978-0-309-43665-6 | DOI 10.17226/23225

AUTHORS

BUY THIS BOOK

FIND RELATED TITLES

Visit the National Academies Press at NAP.edu and login or register to get:

- Access to free PDF downloads of thousands of scientific reports
- 10% off the price of print titles
- Email or social media notifications of new titles related to your interests
- Special offers and discounts



Distribution, posting, or copying of this PDF is strictly prohibited without written permission of the National Academies Press. (Request Permission) Unless otherwise indicated, all materials in this PDF are copyrighted by the National Academy of Sciences.

NCHRP Web-Only Document 82 (Project 4-19 (2)): Appendixes A through F of the Contractor's Final Report

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements

Appendixes A through F of the Contractor's Final Report

Prepared for:

National Cooperative Highway Research Program

TRANSPORTATION RESEARCH BOARD OF THE NATIONAL ACADEMIES

Submitted by:

Thomas D. White Mississippi State University Mississippi State, MS

> John E. Haddock Purdue University West Lafayette, IN

> > March 2006

ACKNOWLEDGMENT

This work was sponsored by the American Association of State Highway and Transportation Officials (AASHTO), in cooperation with the Federal Highway Administration, and was conducted in the National Cooperative Highway Research Program (NCHRP), which is administered by the Transportation Research Board (TRB) of the National Academies.

COPYRIGHT PERMISSION

Authors herein are responsible for the authenticity of their materials and for obtaining written permissions from publishers or persons who own the copyright to any previously published or copyrighted material used herein.

Cooperative Research Programs (CRP) grants permission to reproduce material in this publication for classroom and not-for-profit purposes. Permission is given with the understanding that none of the material will be used to imply TRB, AASHTO, FAA, FHWA, FMCSA, FTA, or Transit Development Corporation endorsement of a particular product, method, or practice. It is expected that those reproducing the material in this document for educational and not-forprofit uses will give appropriate acknowledgment of the source of any reprinted or reproduced material. For other uses of the material, request permission from CRP.

DISCLAIMER

The opinion and conclusions expressed or implied in the report are those of the research agency. They are not necessarily those of the TRB, the National Research Council, AASHTO, or the U.S. Government.

This report has not been edited by TRB.

THE NATIONAL ACADEMIES Advisers to the Nation on Science, Engineering, and Medicine

The **National Academy of Sciences** is a private, nonprofit, self-perpetuating society of distinguished scholars engaged in scientific and engineering research, dedicated to the furtherance of science and technology and to their use for the general welfare. On the authority of the charter granted to it by the Congress in 1863, the Academy has a mandate that requires it to advise the federal government on scientific and technical matters. Dr. Ralph J. Cicerone is president of the National Academy of Sciences.

The **National Academy of Engineering** was established in 1964, under the charter of the National Academy of Sciences, as a parallel organization of outstanding engineers. It is autonomous in its administration and in the selection of its members, sharing with the National Academy of Sciences the responsibility for advising the federal government. The National Academy of Engineering also sponsors engineering programs aimed at meeting national needs, encourages education and research, and recognizes the superior achievements of engineers. Dr. William A. Wulf is president of the National Academy of Engineering.

The **Institute of Medicine** was established in 1970 by the National Academy of Sciences to secure the services of eminent members of appropriate professions in the examination of policy matters pertaining to the health of the public. The Institute acts under the responsibility given to the National Academy of Sciences by its congressional charter to be an adviser to the federal government and, on its own initiative, to identify issues of medical care, research, and education. Dr. Harvey V. Fineberg is president of the Institute of Medicine.

The **National Research Council** was organized by the National Academy of Sciences in 1916 to associate the broad community of science and technology with the Academy's purposes of furthering knowledge and advising the federal government. Functioning in accordance with general policies determined by the Academy, the Council has become the principal operating agency of both the National Academy of Sciences and the National Academy of Engineering in providing services to the government, the public, and the scientific and engineering communities. The Council is administered jointly by both the Academies and the Institute of Medicine. Dr. Ralph J. Cicerone and Dr. William A. Wulf are chair and vice chair, respectively, of the National Research Council.

The **Transportation Research Board** is a division of the National Research Council, which serves the National Academy of Sciences and the National Academy of Engineering. The Board's mission is to promote innovation and progress in transportation through research. In an objective and interdisciplinary setting, the Board facilitates the sharing of information on transportation practice and policy by researchers and practitioners; stimulates research and offers research management services that promote technical excellence; provides expert advice on transportation policy and programs; and disseminates research results broadly and encourages their implementation. The Board's varied activities annually engage more than 5,000 engineers, scientists, and other transportation researchers and practitioners from the public and private sectors and academia, all of whom contribute their expertise in the public interest. The program is supported by state transportation departments, federal agencies including the component administrations of the U.S. Department of Transportation, and other organizations and individuals interested in the development of transportation. **www.TRB.org**

www.national-academies.org

TABLE OF CONTENTS

APPENDIX A PETROGRAPHIC ANALYSIS RESULTS	124
APPENDIX B LABORATORY HMA MIXTURE DESIGN RESULTS	
APPENDIX C AGGREGATE TEST	141
APPENDIX D TEST SECTION CONSTRUCTION AND CONTROL	211
APPENDIX E MOISTURE SUSCEPTIBILITY	224
APPENDIX F BIBLIOGRAPHY	235

APPENDIX A

PETROGRAPHIC ANALYSIS RESULTS

Selection of aggregates for inclusion in the experiments considered mineralogy, level of use throughout the United States, availability, and range of test properties. The goal was to maximize the range of aggregate shape, surface texture, and angularity.

Five coarse and six fine aggregates were included in the study. Coarse aggregates were from dolomite, limestone, granite, gravel, and traprock sources and fine aggregates were from granite, dolomite, gravel, traprock, and two natural sand sources. One of the natural sands has a low UVA value (Natural Sand A) and was obtained from the same source as the gravel. The second natural sand (Natural Sand B) was selected from a different source and has a higher UVA value.

Table A.1 Coarse Dolomite Analysis

Grain size, mm	Mode, mm	Grain SD	Cement, %	Matrix, %	Ground Mass, %	Voids, %	Grain Shape	Alt Grain Shape	Grains, %
0.13	0.085	9.9	7	21.5	28.4	11.7	1.7	8.9	59.9

Table A.2 Traprock Mineral Composition

Mineral Species	Crystal/Grain Size	% of Composition	Comments
Feldspar Plagioclase	Coarse	40-50	Mostly Euhedral
Pyribole highly uralized	Coarse	40-60	Subhedral
Chlorite	Coarse	<5	Irregular masses
Iron Oxides	Coarse	1-2	Irregular masses

Constituent	Percent of Constituent
Clastic	5.67
Clastic (weathered)	9.55
Limestone	22.09
Dolomite	14.03
Dolomite (weathered)	7.76
Quartz	6.87
Quartzite	0.60
Chert	4.78
Mafic	15.82
Felsic	9.85
Oxidized Igneous	2.99
Total	100.00

Table A.3 Gravel and Natural Sand A

Table A.4 Analysis for Granite

Constituent	Percent of Constituent
Granite ¹	85
Schistose rock ²	15

Notes:

- The rock is of granitic composition consisting of approximately 65 percent feldspar (potash + plagioclase), 30 percent quarts, and 10 perent accessory minerals and minor components comprising biotite, muscovite, epidote, and opaque (iron) minerals. The rock is holocrystalline, hypidiomorphic granular intergrown in texture consisting of relatively larger, short, laths of plagioclase (average 1-2 mm) in a matrix of quarts intergrown with microline ina graphic, or mymekitic, texture. Quartz also occurs in finer and coarser-grained pods in which the individual grains have irregular, but not sutured, contacs and predominantly sharp extinction. The grain sizes in the coarse regions are 0.3 to 0.4 mm and in the finer regions approximately 0.03 to 0.04 mm. No fine-grained, distorted, strined, quartzose regions of the potentially alkali-reactive type were observed.
- ² The rock is mica schist, most likely a metamorphosed, hydrothermally altered, basic igneous (metavolcanic) rock. It consists of a dense, compact, interwoven structure of micaceous minerals, chiefly biotite and muscovite with occasional grains of altered plagioclase and occasional void, or vein, filling of calcite. There is little to no quartz in the rock. The rock is predominantly fine-grained (0.8 mm) with occasional large grains up to 0.5 mm maximum dimension. No fine-grained, distorted, strained, quartzose regions of the potentially alkalireactive type were observed.

Table A.5 Limestone Quarry

Devoni	an System
	w Albany Shale
	Shale dark brownish-gray to gray
Noi	th Vernon Limestone
	Limestone, fossiliferous; upper part dark gray, dense, fossiliferous; lower part gray to tan, coarsely crystalline
Jeff	Fersonville Limestone
	Limestone, tan; upper 3,0 ft crystalline, medium bedded, fossiliferous; lower 4.4 ft mottled, fine grained, containing fossil detritus; zone of nodular chert near base
	Limestone in alternating units of tan, brown, and gray color, dolomitic, dense to granular and porous in texture; in places containing breccia and black shale partings; lower part laminated;
	Dolomite and limestone, alternating in few-foot intervals, light tan to brown and gray, banded, chalky, dense in places; lower 1.7 ft gray to dark gray-brown, massive to dense, containing dark-colored bands, stylolitic partings, scattered calcite crystals, and pyrite in vuggy spaces
Get	Limestone, light-gray to light-brown, dolomitic, dense to granular; contains stromatoporoids, trilobites, brachiopoda, cup corals, and other fossils; vugs and solution cavities in lower 3.3 ft (coral zone) neva Dolomite
	Dolomite, brownish-gray to chocolate-brown, fine- grained to saccharoidal, massive; contains small pockets of calcite
	Dolomite, medium-brown to chocolate-brown, granular to saccharoidal, massive; contains large masses of white to yellow coarsely crystalline calcite; dolomitized coral molds and casts in 2 to 3 ft zone 2.6 ft below top of unit; pyrite in places abundant
Siluriar	n System
Lou	isville Limestone
	Limestone, dark medium-gray, fine-grained, crystalline, massive dolomitic; stylolites common

Constituent	Percent of Constituent
Quartz/Quartzite	37.3
Sandstone/Siltstone	24.8
Limestone/Dolomite	33.1
Igneous/Metamorphic	3.5
Chert	0.8
Ironstone	0.5
Total	100.0

Table A.6 Analysis for Natural Sand B

Academy

of

Sciences.

rights

reserved.

All

Copyright

National

APPENDIX B

LABORATORY HMA MIXTURE DESIGN RESULTS

B.1 MIXTURE DESIGNS

A single, unmodified asphalt binder, PG 64-22, was used in all mixtures. This binder represents a typical neat binder grade for much of the United States, and is included in most specifications. It should be noted that the experiments were designed to assess aggregate contribution to HMA mixture performance. The binder properties are summarized in Table B.1.

All mixtures were designed using the Superpave volumetric mixture design method outlined in the Asphalt Institute Manual, SP-2, *Superpave Level I Mix Design*, and subsequent addendum. The mixture design criteria used in the project are shown in Table B.2. The mixture gradations for the coarse-graded, fine-graded, and moisture susceptibility mixtures are shown in Figures B.1, B.2, and B.3, respectively.

PG 64-22 Asphalt Binder				
Property	Test Method	Result	Specifications	
Flash Point (C)	AASHTO T 48	230+	230 Min.	
Rotational Viscosity @ 135C	ASTM D 4402	0.40	3.0 Max.	
(Pa.s)				
G*/sino @64C (kPa)	AASHTO TP 5	1.30	1.0 Min.	
Rolling Thin Film Oven (AASHT	O T 230)			
Mass Loss (%)	AASHTO T 240	0.25	1.0 Max.	
G*/sino @64C (kPa)	AASHTO TP 5	3.05	2.20 Min.	
Pressure Aging Vessel Residue (AASHTO PP 1)				
Pressure Aging Temperature (C)	AASHTO PP 1	-	100	
G*sino @25C (kPa)	AASHTO TP 5	4120	5000 Max.	
Creep Stiffness @ -12C (MPa)	AASHTO TP 1	184	300 Max.	
m-value @ -12C	AASHTO TP 1	0.32	0.30 Min.	

Table B.1	Binder	Properties
-----------	--------	------------

Property	Criteria
N _{initial}	8
N _{design}	100
N _{maximum}	160
Voids @ Optimum Binder Content, %	4.0
VMA, %	14.0
VFA, %	65-75
Dust/Binder Ratio	0.8-1.6
% G _{mm} @ N _{initial}	<89
% G _{mm} @ N _{maximum}	<98

Table B.2 Mixture Design Criteria

B.2 HMA MIXTURE DESIGN FOR RUTTING EXPERIMENTS

B.2.1 Coarse-graded Mixtures

				Ndes			Nmax
Property	Criteria	CA-1 (5.0)	CA-1 (5.5)	CA-1 (6.0)	CA-1 (6.5)	AC_{des}	CA-1 (5.69)
%AC		5.0	5.5	6.0	6.5	5.7	5.7
%Air Voids (V _a)	4.0 %	5.9	4.6	2.9	1.9	4.0	3.0
%VMA	14.0 % Min.	15.2	15.1	14.7	14.9	14.9	14.1
%VFA	65.0 % Min. 75.0 % Max.	61.2	69.6	80.4	87.4	73.4	78.8
Dust/Asphalt Ratio	0.8-1.6 %	0.8	0.7	0.7	0.6	0.7	0.7
Max. Specific Gravity (Gmm)		2.551	2.532	2.512	2.493	2.524	2.524
Bulk Specific Gravity (Gmb)		2.401	2.415	2.440	2.446		2.449
%G _{mm} @ N _{ini}	89.0 % Max.	84.5	85.5	86.8	87.6	86.0	
%G _{mm} @ N _{max}	98.0 % Max.						97.0
Effective Sp. Gravity of Blend (Gse)		2.766	2.766	2.766	2.766	2.766	2.766
Sp. Gravity of Binder (Gb)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gs _b)		2.689	2.689	2.689	2.689	2.689	2.689

				Ndes			Nmax
Property	Criteria	CA-2 (5.5)	CA-2 (6.0)	CA-2 (6.5)	CA-2 (7.0)	*AC _{des}	CA-2 (6.1)
%AC		5.5	6.0	6.5	7.0	6.1	6.1
	10.00	5.5		0.5 3.1	1.9		3.1
%Air Voids (V _a)	4.0 %	5.5	4.1	3.1	1.9	4.0	3.1
%VMA	14.0 % Min.	14.1	13.9	14.1	14.1	14.0	13.2
%VFA	65.0 % Min. 75.0 % Max.	60.9	70.5	78.2	86.8	71.5	76.7
Dust/Asphalt Ratio	0.8-1.6 %	1.1	0.9	0.8	0.8	0.9	0.9
Max. Specific Gravity (Gmm)		2.469	2.451	2.433	2.415	2.447	2.447
Bulk Specific Gravity (G _{mb})		2.333	2.350	2.359	2.371		2.372
%G _{mm} @ N _{ini}	89.0 % Max.	84.1	85.4	86.2	87.1	85.4	
%G _{mm} @ N _{max}	98.0 % Max.						96.9
Effective Sp. Gravity of Blend (Gse)		2.688	2.688	2.688	2.688	2.688	2.688
Sp. Gravity of Binder (G _b)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gsb)		2.566	2.566	2.566	2.566	2.566	2.566

Table B.4 Mixture CA-2: Coarse Limestone (IN) and Natural Sand A (IN)

Table B.5 Mixture CA-3: Coarse Gravel (IN) and Natural Sand A (IN)

				Ndes			Nmax
Property	Criteria	CA-3 (3.5)	CA-3 (4.0)	CA-3 (4.5)	CA-3 (5.0)	*AC _{des}	CA-3 (3.87)
%AC		3.5	4.0	4.5	5.0	3.87	3.87
%Air Voids (V _a)	4.0 %	5.2	3.6	1.7	0.6	4.0	2.9
%VMA	14.0 % Min.	11.2	10.8	10.2	10.3	10.8	9.9
%VFA	65.0 % Min. 75.0 % Max.	53.2	66.6	83.2	94.3	63.6	70.2
Dust/Asphalt Ratio	0.8-1.6 %	1.2	1.0	0.9	0.8	1.1	1.1
Max. Specific Gravity (G _{mm})		2.525	2.506	2.488	2.469	2.511	2.511
Bulk Specific Gravity (G _{mb})		2.393	2.416	2.445	2.455		2.437
%G _{mm} @ N _{ini}	89.0 % Max.	87.5	88.8	90.4	91.4	88.5	
%G _{mm} @ N _{max}	98.0 % Max.						97.1
Effective Sp. Gravity of Blend (Gse)		2.665	2.665	2.665	2.665	2.665	2.665
Sp. Gravity of Binder (G _b)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gsb)		2.600	2.600	2.600	2.600	2.600	2.600

				Ndes			Nmax
Property	Criteria	CA-4 (5.0)	CA-4 (5.5)	CA-4 (6.0)	CA-4 (6.5)	*AC _{des}	CA-4 (5.8)
****		5.0			0.5	5.04	5.0
%AC		5.0	5.5	6.0	6.5	5.81	5.8
%Air Voids (Va)	4.0 %	5.8	4.6	3.5	2.5	4.0	2.9
%VMA	14.0 % Min.	16.0	16.0	16.1	16.3	16.0	15.1
%VFA	65.0 % Min. 75.0 % Max.	63.5	71.0	78.1	84.5	75.1	80.8
Dust/Asphalt Ratio	0.8-1.6 %	0.8	0.7	0.6	0.6	0.7	0.6
Max. Specific Gravity (Gmm)		2.491	2.472	2.454	2.436	2.461	2.461
Bulk Specific Gravity (Gmb)		2.345	2.357	2.368	2.375		2.389
%G _{mm} @ N _{ini}	89.0 % Max.	85.8	86.6	87.3	88.1	87.0	
%G _{mm} @ N _{max}	98.0 % Max.						97.1
Effective Sp. Gravity of Blend (Gse)		2.691	2.691	2.691	2.691	2.691	2.691
Sp. Gravity of Binder (Gb)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gsb)		2.652	2.652	2.652	2.652	2.652	2.652

Table B.6 Mixture CA-4: Coarse Granite (NC) and Natural Sand A (IN)

*The volumetric properties of mixtures with design asphalt content and compacted at N design are computed using regression analysis

				Ndes			Nmax
Property	Criteria	CA-5 (4.0)	CA-5 (4.5)	CA-5 (5.0)	CA-5 (5.5)	*AC _{des}	CA-5 (3.72)
%AC		4.0	4.5	5.0	5.5	4.8	4.8
%Air Voids (V _a)	4.0 %	6.0	4.6	3.5	1.9	4.0	3.0
%VMA	14.0 % Min.	14.3	14.3	14.4	14.2	14.3	13.6
%VFA	65.0 % Min. 75.0 % Max.	58.4	67.6	75.6	86.5	72.0	77.5
Dust/Asphalt Ratio	0.8-1.6 %	0.7	0.6	0.6	0.5	0.6	0.6
Max. Specific Gravity (G _{mm})		2.664	2.643	2.621	2.600	2.630	2.630
Bulk Specific Gravity (G _{mb})		2.506	2.520	2.529	2.550		2.549
%G _{mm} @ N _{ini}	89.0 % Max.	86.5	87.6	88.7	89.9	88.2	
%G _{mm} @ N _{max}	98.0 % Max.						97.0
Effective Sp. Gravity of Blend (Gse)		2.853	2.853	2.853	2.853	2.853	2.853
Sp. Gravity of Binder (Gb)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gs _b)		2.808	2.808	2.808	2.808	2.808	2.808

Table B.7 Mixture CA-5: Coarse Traprock (VA) and Natural Sand A (IN)

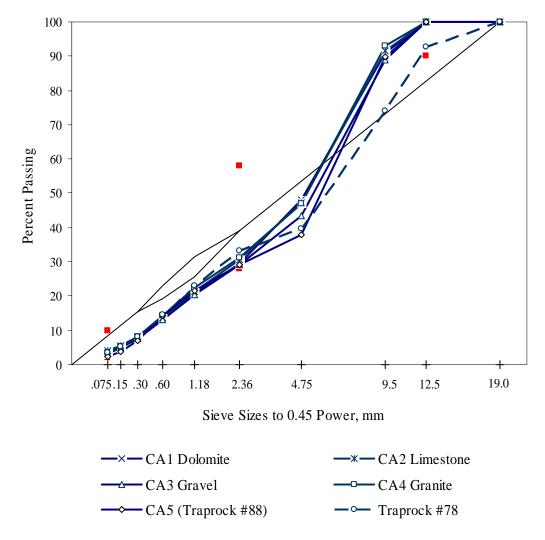


Figure B.1 Coarse-Graded Mixture Gradations

B.2.2 Fine-graded Mixtures

			Ndes			Nmax
Criteria	FA-1A (5.0)	FA-1A (5.5)	FA-1A (6.5)	FA-1 (6.0)	*AC _{des}	FA-1 (6.0)
	5.0	5.5	6.5	6.0	6.0	6.0
4.0 %	7.2	5.5	2.7	4.1	4.0	3.3
14.0 % Min.	15.9	15.5	15.1	15.3	15.3	14.6
65.0 % Min. 75.0 % Max	55.0	64.5	82.1	73.3	74.0	77.4
0.8-1.6 %	0.6	0.5	0.4	0.4	0.4	0.4
	2.474	2.455	2.420	2.438	2.438	2.438
	2.296	2.321	2.354	2.338		2.357
89.0 % Max.	87.9	89.4	92.0	90.4	90.7	
98.0 % Max.						96.7
	2.671	2.671	2.671	2.671	2.671	2.671
	1.030	1.030	1.030	1.030	1.030	1.030
	2.594	2.594	2.594	2.594	2.594	2.594
	4.0 % 14.0 % Min. 65.0 % Min. 75.0 % Max. 0.8-1.6 % 89.0 % Max. 98.0 % Max. 	4.0 % 5.0 4.0 % 7.2 14.0 % Min. 15.9 65.0 % Min. 55.0 75.0 % Max. 0.6 2.474 2.296 89.0 % Max. 87.9 98.0 % Max. 2.671 1.030	4.0 % 5.0 5.5 4.0 % Min. 15.9 15.5 65.0 % Min. 55.0 64.5 75.0 % Max. 0.6 0.5 2.474 2.455 2.296 2.321 89.0 % Max. 87.9 89.4 98.0 % Max. 2.671 2.671 1.030 1.030 1.030	Criteria FA-1A (5.0) FA-1A (5.5) FA-1A (6.5) 4.0 % 5.0 5.5 6.5 4.0 % 7.2 5.5 2.7 14.0 % Min. 15.9 15.5 15.1 65.0 % Min. 55.0 64.5 82.1 75.0 % Max. 0.6 0.5 0.4 2.474 2.455 2.420 2.296 2.321 2.354 89.0 % Max. 87.9 89.4 92.0 98.0 % Max. 2.671 2.671 2.671 2.671 2.671 2.671 1.030	Criteria FA-1A (5.0) FA-1A (5.5) FA-1A (6.5) FA-1 (6.0) 4.0 % 5.0 5.5 6.5 6.0 4.0 % 7.2 5.5 2.7 4.1 14.0 % Min. 15.9 15.5 15.1 15.3 65.0 % Min. 55.0 64.5 82.1 73.3 0.8-1.6 % 0.6 0.5 0.4 0.4 2.474 2.455 2.420 2.438 2.296 2.321 2.354 2.338 89.0 % Max. 87.9 89.4 92.0 90.4 98.0 % Max. 2.671 2.671 2.671 2.671 2.671 2.671 2.671 2.671 1.030 1.030 1.030 1.030	Criteria FA-1A (5.0) FA-1A (5.5) FA-1A (6.5) FA-1 (6.0) *AC _{des} 4.0 % 5.0 5.5 6.5 6.0 6.0 14.0 % 7.2 5.5 2.7 4.1 4.0 14.0 % Min. 15.9 15.5 15.1 15.3 15.3 65.0 % Min. 55.0 64.5 82.1 73.3 74.0 0.8-1.6 % 0.6 0.5 0.4 0.4 0.4 2.474 2.455 2.420 2.438 2.438 2.296 2.321 2.354 2.338 89.0 % Max. 87.9 89.4 92.0 90.4 90.7 98.0 % Max. 2.671 2.671 2.671 2.671 2.671 2.671 2.671 2.671 2.671 2.671 2.671 1.030 1.030 1.030 1.030 1.030 1.030

Table B.8 Mixture FA-1: Natural Sand A (IN) and Coarse Gravel (IN)

*The volumetric properties of mixtures with design asphalt content and compacted at N design are computed using regression analysis

				Ndes			Nmax
Property	Criteria	FA-2B (6.7)	FA-2B (5.7)	FA-2B (5.2)	FA-2B (6.2)	*AC _{des}	FA-2 (5.71)
%AC		6.7	5.7	5.2	6.2	5.71	5.7
%Air Voids (V _a)	4.0 %	1.3	4.2	5.4	2.3	4.0	3.1
%VMA	14.0 % Min.	15.8	16.2	16.2	15.6	16.2	15.2
%VFA	65.0 % Min. 75.0 % Max.	91.8	73.9	66.7	85.1	75.2	79.9
Dust/Asphalt Ratio	0.8-1.6 %	0.6	0.7	0.8	0.7	0.7	0.7
Max. Specific Gravity (G _{mm})		2.412	2.447	2.465	2.429	2.447	2.447
Bulk Specific Gravity (G _{mb})		2.381	2.343	2.332	2.373		2.372
%G _{mm} @ N _{ini}	89.0 % Max.	90.4	88.2	87.1	89.6	88.3	
%G _{mm} @ N _{max}	98.0 % Max.						96.9
Effective Sp. Gravity of Blend (Gse)		2.669	2.669	2.669	2.669	2.669	2.669
Sp. Gravity of Binder (G _b)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gs_b)		2.638	2.638	2.638	2.638	2.638	2.638

Table B.9 Mixture FA-2: Crushed gravel sand (IN) and Coarse Gravel (IN)

				Ndes			Nmax
Property	Criteria	FA-3 (5.0)	FA-3 (5.5)	FA-3 (6.0)	FA-3 (6.5)	*AC _{des}	FA-3 (5.84)
%AC		5.0	5.5	6.0	6.5	5.8	5.8
%Air Voids (Va)	4.0 %	6.6	4.8	3.5	2.1	4.0	2.9
%VMA	14.0 % Min.	15.6	15.1	15.0	14.8	15.0	14.1
%VFA	65.0 % Min. 75.0 % Max.	57.8	68.0	76.6	85.6	73.6	79.7
Dust/Asphalt Ratio	0.8-1.6 %	0.9	0.8	0.7	0.7	0.8	0.8
Max. Specific Gravity (G _{mm})		2.475	2.456	2.439	2.421	2.444	2.444
Bulk Specific Gravity (Gmb)		2.312	2.338	2.353	2.369		2.374
%G _{mm} @ N _{ini}	89.0 % Max.	86.8	88.2	89.2	90.4	88.9	
%G _{mm} @ N _{max}	98.0 % Max.						97.1
Effective Sp. Gravity of Blend (Gse)		2.672	2.672	2.672	2.672	2.672	2.672
Sp. Gravity of Binder (G _b)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gsb)		2.602	2.602	2.602	2.602	2.602	2.602

Table B.10 Mixture FA-3: Natural Sand B (OH) and Coarse Gravel (IN)

*The volumetric properties of mixtures with design asphalt content and compacted at N design are computed using regression analysis

				Ndes			Nmax
Property	Criteria	FA-4 (4.0)	FA-4 (4.5)	FA-4 (5.0)	FA-4 (6.0)	*AC _{des}	FA-4 (4.94)
%AC		4.0	4.5	5.0	6.0	4.9	4.9
%Air Voids (V _a)	4.0 %	7.2	5.3	3.5	0.8	4.0	2.7
%VMA	14.0 % Min.	15.4	14.7	14.2	13.9	14.3	13.3
%VFA	65.0 % Min. 75.0 % Max.	53.0	63.9	75.1	94.3	73.0	79.6
Dust/Asphalt Ratio	0.8-1.6 %	2.3	2.1	1.8	1.5	1.9	1.9
Max. Specific Gravity (Gmm)		2.494	2.476	2.457	2.422	2.460	2.460
Bulk Specific Gravity (Gmb)		2.314	2.344	2.370	2.403		2.393
%G _{mm} @ N _{ini}	89.0 % Max.	85.5	87.0	88.3	91.0	88.1	
%Gmm @ Nmax	98.0 % Max.						97.3
Effective Sp. Gravity of Blend (Gse)		2.651	2.651	2.651	2.651	2.651	2.651
Sp. Gravity of Binder (Gb)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gsb)		2.625	2.625	2.625	2.625	2.625	2.625

Table B.11 Mixture FA-4: Granite Sand (NC) and Coarse Gravel (IN)

*The volumetric properties of mixtures with design asphalt content and compacted at N design are computed using regression analysis

				Ndes			Nmax
Property	Criteria	FA-5N-03 (6.0)	FA-5N-03 (6.5)	FA-5N-03 (7.0)	FA-5N-03 (7.5)	*AC _{des}	FA5N-03 (6.3)
%AC		6.0	6.5	7.0	7.5	6.3	6.3
%Air Voids (V _a)	4.0 %	4.8	3.5	2.0	0.3	4.0	2.6
%VMA %VFA	14.0 % Min. 65.0 % Min. 75.0 % Max.	15.8 69.8	15.8 77.6	15.5 87.4	15.2 97.8	15.8 74.7	14.5 82.3
Dust/Asphalt Ratio	0.8-1.6 %	0.7	0.7	0.6	0.6	0.7	0.7
Max. Specific Gravity (Gmm)		2.465	2.446	2.429	2.411	2.454	2.454
Bulk Specific Gravity (G _{mb})		2.347	2.360	2.381	2.403		2.391
%G _{mm} @ N _{ini}	89.0 % Max.	87.0	88.0	88.8	90.2	87.5	
%G _{mm} @ N _{max}	98.0 % Max.						97.4
Effective Sp. Gravity of Blend (G_{se})		2.705	2.705	2.705	2.705	2.705	2.705
Sp. Gravity of Binder (G _b)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gs _b)		2.620	2.620	2.620	2.620	2.620	2.620

Table B.12 Mixture FA-5: Dolomite Sand (IN) and Coarse Gravel (IN)

*The volumetric properties of mixtures with design asphalt content and compacted at N design are computed using regression analysis

				Ndes			Nmax
Property	Criteria	FA-6 (4.0)	FA-6 (4.5)	FA-6 (5.0)	FA-6 (6.0)	*AC _{des}	FA-6 (4.9)
%AC		4.0	4.5	5.0	6.0	4.9	4.9
%Air Voids (Va)	4.0 %	7.2	5.3	3.5	0.5	4.0	2.8
%VMA	14.0 % Min.	15.4	14.8	14.3	14.0	14.4	13.4
%VFA	65.0 % Min. 75.0 % Max.	53.0	63.9	75.6	96.1	72.2	79.2
Dust/Asphalt Ratio	0.8-1.6 %	2.0	1.8	1.6	1.3	1.6	1.6
Max. Specific Gravity (Gmm)		2.658	2.636	2.615	2.573	2.619	2.619
Bulk Specific Gravity (Gmb)		2.466	2.496	2.524	2.559		2.546
%G _{mm} @ N _{ini}	89.0 % Max.	85.3	86.5	87.9	90.5	87.7	
%G _{mm} @ N _{max}	98.0 % Max.						97.2
Effective Sp. Gravity of Blend (Gse)		2.845	2.845	2.845	2.845	2.845	2.845
Sp. Gravity of Binder (Gb)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gsb)		2.797	2.797	2.797	2.797	2.797	2.797
Sp. Gravity of Aggregate (GSb)		2.797	2.191	2.797	2.191	2.191	2.79

Table B.13 Mixture FA-6: Traprock Sand (VA) and Coarse Gravel (IN

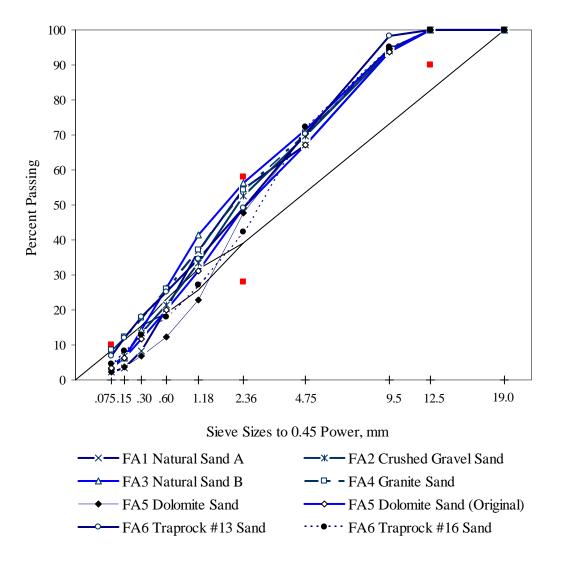


Figure B.2 Fine-Graded Mixture Gradations

B.3 HMA MIXTURE DESIGN FOR MOISTURE SUSCEPTIBILITY EXPERIMENTS

B.3.1 Fine-graded Mixtures

Table B.14 Mixture FAM-1: Natural Sand A	(IN) and Coarse	Dolomite	(IN)
Tuble Diff infiniture I finit fit futurul build fit	(** *	, und Course		111/

				Ndes			Nmax
Property	Criteria	FAM-1 (5.0)	FAM-1 (5.5)	FAM-1 (6.0)	FAM-1 (6.5)	*AC _{des}	FAM-1 (6.1)
%AC		5.0	5.5	6.0	6.5	6.1	6.1
%Air Voids (V _a)	4.0 %	7.3	5.7	4.3	2.6	4.0	3.1
%VMA	14.0 % Min.	16.2	15.8	15.7	15.3	15.6	14.7
%VFA	65.0 % Min. 75.0 % Max.	55.0	63.9	72.6	82.8	74.4	78.9
Dust/Asphalt Ratio	0.8-1.6 %	0.9	0.8	0.7	0.6	0.7	0.7
Max. Specific Gravity (Gmm)		2.521	2.502	2.483	2.464	2.481	2.481
Bulk Specific Gravity (Gmb)		2.337	2.359	2.376	2.399		2.404
%G _{mm} @ N _{ini}	89.0 % Max.	87.1	88.4	89.8	91.4	90.1	
%G _{mm} @ N _{max}	98.0 % Max.						96.9
Effective Sp. Gravity of Blend (Gse)		2.728	2.728	2.728	2.728	2.728	2.728
Sp. Gravity of Binder (G _b)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gs_b)		2.648	2.648	2.648	2.648	2.648	2.648

*The volumetric properties of mixtures with design asphalt content and compacted at N design are computed using regression analysis

Table B.15 Mixture FAM-2: Crushed gravel sand (IN) and Coarse Dolomite (IN)

				Ndes			Nmax
Property	Criteria	FAM-2 (5.5)	FAM-2 (6.0)	FAM-2 (6.5)	FAM-2 (7.0)	*AC _{des}	FAM-2 (6.4)
%AC		5.5	6.0	6.5	7.0	6.35	6.4
							_
%Air Voids (V _a)	4.0 %	6.7	4.9	3.5	2.2	4.0	2.8
%VMA	14.0 % Min.	17.4	16.9	16.8	16.7	16.8	15.9
%VFA	65.0 % Min. 75.0 % Max.	61.7	71.0	79.1	87.0	76.4	82.3
Dust/Asphalt Ratio	0.8-1.6 %	1.1	1.0	0.9	0.8	0.9	0.9
Max. Specific Gravity (G _{mm})		2.517	2.498	2.479	2.461	2.485	2.485
Bulk Specific Gravity (Gmb)		2.349	2.376	2.392	2.407		2.415
%G _{mm} @ N _{ini}	89.0 % Max.	85.6	87.0	87.9	88.9	87.6	
%G _{mm} @ N _{max}	98.0 % Max.						97.2
Effective Sp. Gravity of Blend (Gse)		2.748	2.748	2.748	2.748	2.748	2.748
Sp. Gravity of Binder (Gb)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gs _b)		2.687	2.687	2.687	2.687	2.687	2.687

				Ndes			Nmax
Property	Criteria	FAM-3 (5.0)	FAM-3 (5.5)	FAM-3 (6.0)	FAM-3 (6.5)	*AC _{des}	FAM-3 (5.4)
%AC		5.0	5.5	6.0	6.5	5.41	5.4
%Air Voids (V _a)	4.0 %	5.4	3.6	2.0	0.9	4.0	2.8
%VMA	14.0 % Min.	15.8	15.2	14.9	15.1	15.3	14.4
%VFA	65.0 % Min. 75.0 % Max.	65.7	76.7	86.8	94.0	74.3	80.3
Dust/Asphalt Ratio	0.8-1.6 %	1.9	1.7	1.5	1.4	1.7	1.7
Max. Specific Gravity (Gmm)		2.503	2.485	2.466	2.448	2.488	2.488
Bulk Specific Gravity (Gmb)		2.368	2.396	2.418	2.426		2.417
%G _{mm} @ N _{ini}	89.0 % Max.	86.3	87.8	89.2	90.5	87.5	
%G _{mm} @ Nmax	98.0 % Max.						97.2
Effective Sp. Gravity of Blend (Gse)		2.707	2.707	2.707	2.707	2.707	2.707
Sp. Gravity of Binder (G _b)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gs _b)		2.671	2.671	2.671	2.671	2.671	2.671

Table B.16 Mixture FAM-3: Granite Sand (NC) and Coarse Dolomite (IN)

*The volumetric properties of mixtures with design asphalt content and compacted at N design are computed using regression analysis

				Ndes			Nmax
		FAM-4(02)	FAM-4(02)	FAM-4(02)	FAM-4(02)		FAM-4(02)
Property	Criteria	(5.0)	(5.5)	(6.0)	(6.5)	*AC _{des}	(5.3)
%AC		5.0	5.5	6.0	6.5	5.3	5.3
%Air Voids (Va)	4.0 %	4.9	3.3	1.3	0.5	4.0	2.7
%VMA	14.0 % Min.	15.2	14.9	14.3	14.8	15.0	13.8
%VFA	65.0 % Min. 75.0 % Max.	67.6	77.7	91.0	96.7	73.3	80.4
Dust/Asphalt Ratio	0.8-1.6 %	1.5	1.4	1.2	1.1	1.4	1.4
Max. Specific Gravity (Gmm)		2.662	2.640	2.618	2.597	2.650	2.650
Bulk Specific Gravity (Gmb)		2.531	2.552	2.585	2.584		2.578
%Gmm @ Nini	89.0 % Max.	86.1	87.1	88.7	89.9	86.7	
%G _{mm} @ N _{max}	98.0 % Max.						97.3
Effective Sp. Gravity of Blend (Gse)		2.904	2.904	2.904	2.904	2.904	2.904
Sp. Gravity of Binder (G _b)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gs _b)		2.835	2.835	2.835	2.835	2.835	2.835

Table B.17 Mixture FAM-4: Traprock Sand (VA) and Coarse Dolomite (IN)

*The volumetric properties of mixtures with design asphalt content and compacted at N design are computed using regression analysis

				Ndes			Nmax
Property	Criteria	FAM-5 (5.5)	FAM-5 (6.0)	FAM-5 (6.5)	FAM-5 (7.0)	*AC _{des}	FAM-5 (6.1)
%AC		5.5	6.0	6.5	7.0	6.1	6.1
%Air Voids (V _a)	4.0 %	5.8	4.1	2.9	1.6	4.0	3.2
%VMA	14.0 % Min.	16.3	15.9	15.9	15.8	15.9	15.2
%VFA	65.0 % Min.	64.5	74.3	81.7	90.1	75.2	79.2
Dust/Asphalt Ratio	75.0 % Max. 0.8-1.6 %	0.8	0.7	0.7	0.6	0.7	0.7
Max. Specific Gravity (G _{mm})		2.491	2.473	2.454	2.436	2.469	2.469
Bulk Specific Gravity (Gmb)		2.347	2.372	2.383	2.398		2.394
%G _{mm} @ N _{ini}	89.0 % Max.	87.5	88.8	89.8	91.0	88.9	
%G _{mm} @ Nmax	98.0 % Max.						96.8
Effective Sp. Gravity of Blend (Gse)		2.715	2.715	2.715	2.715	2.715	2.715
Sp. Gravity of Binder (G _b)		1.030	1.030	1.030	1.030	1.030	1.030
Sp. Gravity of Aggregate (Gsb)		2.650	2.650	2.650	2.650	2.650	2.650

Table B.18 Mixture FAM-5: Natural Sand B (OH) and Coarse Dolomite (IN)

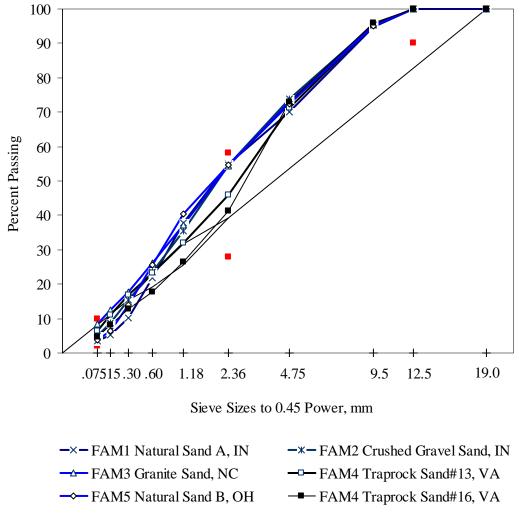


Figure B.3 Moisture Susceptibility Mixture Gradations

APPENDIX C

AGGREGATE TEST

C.1 FLAT OR ELONGATED PARTICLE TEST (ASTM D4791) – LABORATORY MIX DESIGN SOURCE

C.1.1 Dolomite Coarse Aggregate (Indiana)

Table C.1 Flat or Elongated Particle 2:1 Ratio

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Faiticle Size		g	g	g	g	%	%	%	%
	Trial 1	94.0	43.9	336.4	474.3	19.8	9.3	29.1	70.9
-12.5mm + 9.5mm	Trial 2	106.9	37.6	379.6	524.1	20.4	7.2	27.6	72.4
	Trial 3	18.1	0.0	127.1	145.2	12.5	0.0	12.5	87.5
	Trial 1	75.2	71.4	125.8	272.4	27.6	26.2	53.8	46.2
-9.5mm + 4.75mm	Trial 2	28.6	15.3	54.0	97.9	29.2	15.6	44.8	55.2
	Trial 3	28.7	16.7	45.1	90.5	31.7	18.5	50.2	49.8

Summary								
	Component		Average					
	Component	Flat	Elongated	FOE	Non			
Particle Size	%	%	%	%				
-12.5mm + 9.5mm 13.4		17.6	5.5	23.0	77.0			
-9.5mm + 4.75mm 61.9		29.5	20.1	49.6	50.4			
We	27.4	17.5	44.9	55.1				

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
		g	g	g	50	%	%	%	%
	Trial 1	7.3	0.0	346.2	353.5	2.1	0.0	2.1	97.9
-12.5mm + 9.5mm	Trial 2	3.0	0.0	334.1	337.1	0.9	0.0	0.9	99.1
	Trial 3	2.9	0.0	342	344.9	0.8	0.0	0.8	99.2
	Trial 1	5.6	0.0	99.6	105.2	5.3	0.0	5.3	94.7
-9.5mm + 4.75mm	Trial 2	4.3	0.0	91.2	95.5	4.5	0.0	4.5	95.5
	Trial 3	12.7	0.0	191.4	204.1	6.2	0.0	6.2	93.8

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

Table C.2 Flat or Elongated Particle 3:1 Ratio

Summary								
	Component	Average						
	Component	Flat	Elongated	FOE	Non			
Particle Size %		%	%	%	%			
-12.5mm + 9.5mm 13.4		1.3	0.0	1.3	98.7			
-9.5mm + 4.75mm	61.9	5.3	0.0	5.3	94.7			
Weighte	4.6	0.0	4.6	95.4				

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	g	g	%	%
	Trial 1	8.7	344.8	353.5	2.5	97.5
-12.5mm + 9.5mm	Trial 2	18.0	319.0	337.0	5.3	94.7
	Trial 3	21.2	323.8	345.0	6.1	93.9
	Trial 1	28.5	76.9	105.4	27.0	73.0
-9.5mm + 4.75mm	Trial 2	22.2	73.5	95.7	23.2	76.8
	Trial 3	53.3	150.8	204.1	26.1	73.9

Table C.3 Flat and Elongated I	Particle 3:1 Ratio
--------------------------------	--------------------

	Summary				
		Average			
	Component	Flat and Elongated	Non		
Particle Size	%	%	%		
-12.5mm + 9.5mm	13.4	4.6	95.4		
-9.5mm + 4.75mm	61.9	25.5	74.5		
Weighte	Weighted Average =				

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
		g	g	g	50	%	%	%	%
	Trial 1	0.0	0.0	210.4	210.4	0.0	0.0	0.0	100.0
-12.5mm + 9.5mm	Trial 2	0.0	0.0	187.5	187.5	0.0	0.0	0.0	100.0
	Trial 3	0.0	0.0	198.6	198.6	0.0	0.0	0.0	100.0
	Trial 1	4.3	0.0	229.9	234.2	1.8	0.0	1.8	98.2
-9.5mm + 4.75mm	Trial 2	1.2	0.0	225.9	227.1	0.5	0.0	0.5	99.5
	Trial 3	0.0	0.0	90.5	90.5	0.0	0.0	0.0	100.0

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

Table C.4 Flat or Elongated Particle 5:1 Ratio

		Summary	•					
	Component		Average					
	Component	Flat	Elongated	FOE	Non			
Particle Size	%	%	%	%	%			
-12.5mm + 9.5mm	13.4	0.0	0.0	0.0	100.0			
-9.5mm + 4.75mm	61.9	0.8	0.0	0.8	99.2			
Weighte	Weighted Average =		0.0	0.7	99.3			

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	g	g	%	%
	Trial 1	0.9	266.3	267.2	0.3	99.7
-12.5mm + 9.5mm	Trial 2	1.1	278.8	279.9	0.4	99.6
	Trial 3	2.8	269.9	272.7	1.0	99.0
	Trial 1	7.7	102.1	109.8	7.0	93.0
-9.5mm + 4.75mm	Trial 2	9.1	93.9	103	8.8	91.2
	Trial 3	6.3	87.4	93.7	6.7	93.3

Table C.5 Flat and Elongated Particle 5:1 Ratio

	Summary								
		Average							
	Component	Flat and							
		Elongated	Non						
Particle Size	%	%	%						
-12.5mm + 9.5mm	13.4	0.6	99.4						
-9.5mm + 4.75mm	61.9	7.5	92.5						
Weighte	ed Average =	6.3	93.7						

C.1.2 Limestone Coarse Aggregate (Indiana)

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Faiticle Size		g	g	g	g	%	%	%	%
-12.5mm + 9.5mm	Trial 1	87.0	34.6	155.0	276.6	31.5	12.5	44.0	56.0
	Trial 2	72.3	38.9	176.0	287.2	25.2	13.5	38.7	61.3
	Trial 3	96.0	41.1	244.6	381.7	25.2	10.8	35.9	64.1
-9.5mm + 4.75mm	Trial 1	54.3	39.1	78.1	171.5	31.7	22.8	54.5	45.5
	Trial 2	39.4	25.2	78.6	143.2	27.5	17.6	45.1	54.9
	Trial 3	19.8	14.4	33.2	67.4	29.4	21.4	50.7	49.3

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

Table C.6 Flat or Elongated Particle 2:1 Ratio

		Summary						
	Component		Average					
	Component	Flat	Elongated	FOE	Non			
Particle Size	%	%	%	%	%			
-12.5mm + 9.5mm	12.1	27.3	12.3	39.5	60.5			
-9.5mm + 4.75mm	63.2	29.5	20.6	50.1	49.9			
Weighted Average =		29.2	19.3	48.4	51.6			

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
I alticle Size		g	g	g	g	%	%	%	%
-12.5mm + 9.5mm	Trial 1	7.3	0.0	285.7	293.0	2.5	0.0	2.5	97.5
	Trial 2	7.8	0.0	296.5	304.3	2.6	0.0	2.6	97.4
	Trial 3	12.1	0.0	280.3	292.4	4.1	0.0	4.1	95.9
	Trial 1	13.6	0.0	167.0	180.6	7.5	0.0	7.5	92.5
-9.5mm + 4.75mm	Trial 2	5.1	0.0	87.0	92.1	5.5	0.0	5.5	94.5
	Trial 3	6.1	0.0	85.9	92.0	6.6	0.0	6.6	93.4

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

Table C.7 Flat or Elongated Particle 3:1 Ratio

		Summary						
	Component	Average						
	Component	Flat	Elongated	FOE	Non			
Particle Size	%	%	%	%	%			
-12.5mm + 9.5mm	12.1	3.1	0.0	3.1	96.9			
-9.5mm + 4.75mm	63.2	6.6	0.0	6.6	93.4			
Weighte	Weighted Average =			6.0	94.0			

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	g	g	%	%
	Trial 1	38.5	254.6	293.1	13.1	86.9
-12.5mm + 9.5mm	Trial 2	24.7	279.5	304.2	8.1	91.9
	Trial 3	35.2	257.1	292.3	12.0	88.0
	Trial 1	63.7	116.9	180.6	35.3	64.7
-9.5mm + 4.75mm	Trial 2	26.1	66.0	92.1	28.3	71.7
	Trial 3	27.6	64.4	92.0	30.0	70.0

Table C.8 Flat and Elongated Particle 3:1 Ratio

	Summary			
		Average		
	Component	Flat and Elongated	Non	
Particle Size	%	%	%	
-12.5mm + 9.5mm	12.1	11.1	88.9	
-9.5mm + 4.75mm	63.2	31.2	68.8	
Weighted	l Average =	28.0	72.0	

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
I diticle Size		g	g	g	g	%	%	%	%
-12.5mm + 9.5mm	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	2.6	0.0	194.2	196.8	1.3	0.0	1.3	98.7
-9.5mm + 4.75mm	Trial 2	1.9	1.0	143.0	146.2	1.3	0.9	2.2	97.8
	Trial 3	1.9	0.0	172.2	174.1	1.1	0.0	1.1	98.9

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

Table C.9 Flat or Elongated Particle 5:1 Ratio

Summary										
	Component	Average								
	Component	Flat	Elongated	FOE	Non					
Particle Size	%	%	%	%	%					
-12.5mm + 9.5mm	12.1	0.0	0.0	0.0	100.0					
-9.5mm + 4.75mm	63.2	1.2	0.3	1.5	98.5					
Weighte	Weighted Average =		0.2	1.3	98.7					

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	g	g	%	%
	Trial 1	3.5	219.7	223.2	1.6	98.4
-12.5mm + 9.5mm	Trial 2	6.0	216.3	222.3	2.7	97.3
	Trial 3	3.4	224.5	227.9	1.5	98.5
	Trial 1	10.2	99.3	109.5	9.3	90.7
-9.5mm + 4.75mm	Trial 2	8.0	98.0	106.0	7.5	92.5
	Trial 3	11.1	89.1	100.2	11.1	88.9

Table C.10 Flat and Elongated Particle 5:1 Ratio)
--	---

	Summary										
		Aver	age								
	Component	Flat and Elongated	Non								
Particle Size	%	%	%								
-12.5mm + 9.5mm	12.1	1.9	98.1								
-9.5mm + 4.75mm	63.2	9.3	90.7								
Weight	ed Average =	8.1	91.9								

C.1.3 Gravel Coarse Aggregate (Indiana)

Dortiolo Sizo		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Particle Size		G	g	g	g	%	%	%	%
	Trial 1	45.8	7.0	234.9	287.7	15.9	2.4	18.4	81.6
-12.5mm + 9.5mm	Trial 2	51.9	11.4	241.7	305.0	17.0	3.7	20.8	79.2
	Trial 3	44.5	0.0	191.4	235.9	18.9	0.0	18.9	81.1
	Trial 1	75.7	34.6	258.9	369.2	20.5	9.4	29.9	70.1
-9.5mm + 4.75mm	Trial 2	97.0	33.1	339.1	469.2	20.7	7.1	27.7	72.3
	Trial 3	30.9	8.8	71.5	111.2	27.8	7.9	35.7	64.3

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

Table C.11 Flat or Elongated Particle 2:1 Ratio

Summary										
	Component		Averag	ge						
	Component	Flat	Elongated	FOE	Non					
Particle Size	%	%	%	%	%					
-12.5mm + 9.5mm	12.5mm + 9.5mm 15.9		2.1	19.3	80.7					
-9.5mm + 4.75mm	66.0	23.0	8.1	31.1	68.9					
Weight	21.9	6.9	28.8	71.2						

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
		G	50	g	g	%	%	%	%
	Trial 1	4.0	0.0	187.5	191.5	2.1	0.0	2.1	97.9
-12.5mm + 9.5mm	Trial 2	2.4	0.0	204.7	207.1	1.2	0.0	1.2	98.8
	Trial 3	4.7	0.0	193.4	198.1	2.4	0.0	2.4	97.6
	Trial 1	3.8	0.0	155.3	159.1	2.4	0.0	2.4	97.6
-9.5mm + 4.75mm	Trial 2	5.5	0.0	123.1	128.6	4.3	0.0	4.3	95.7
	Trial 3	2.3	0.0	140.8	143.1	1.6	0.0	1.6	98.4

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

Table C.12 Flat or Elongated Particle 3:1 Ratio

Summary										
	Component		Averag	je						
	Component	Flat	Elongated	FOE	Non					
Particle Size	%	%	%	%	%					
-12.5mm + 9.5mm	15.9	1.9	0.0	1.9	98.1					
-9.5mm + 4.75mm	66.0	2.8	0.0	2.8	97.2					
Weight	2.6	0.0	2.6	97.4						

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		G	g	g	%	%
	Trial 1	18.1	173.5	191.6	9.4	90.6
-12.5mm + 9.5mm	Trial 2	10.8	196.4	207.2	5.2	94.8
	Trial 3	16.9	181.1	198.0	8.5	91.5
	Trial 1	22.8	136.4	159.2	14.3	85.7
-9.5mm + 4.75mm	Trial 2	23.5	105.0	128.5	18.3	81.7
	Trial 3	15.6	127.4	143.0	10.9	89.1

Summary										
		Aver	age							
	Component	Flat and								
		Elongated	Non							
Particle Size	%	%	%							
-12.5mm + 9.5mm	15.9	7.7	92.3							
-9.5mm + 4.75mm	66.0	14.5	85.5							
Weight	Weighted Average =									

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
r article Size		g	g	g	g	%	%	%	%
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-12.5mm + 9.5mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-9.5mm + 4.75mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

Table C.14 Flat or Elongated Particle 5:1 Ratio

Summary					
	Component	Average			
	Component	Flat	Elongated	FOE	Non
Particle Size	%	%	%	%	%
-12.5mm + 9.5mm	15.9	0.0	0.0	0.0	100.0
-9.5mm + 4.75mm	66.0	0.0	0.0	0.0	100.0
Weighted Average =		0.0	0.0	0.0	100.0

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	g	g	%	%
	Trial 1	2.2	280.8	283	0.8	99.2
-12.5mm + 9.5mm	Trial 2	2.2	249.9	252.1	0.9	99.1
	Trial 3	0.0	348.0	348.0	0.0	100.0
	Trial 1	2.5	124.4	126.9	2.0	98.0
-9.5mm + 4.75mm	Trial 2	3.4	127.5	130.9	2.6	97.4
	Trial 3	2.2	129.1	131.3	1.7	98.3

Table C.15 Flat and Elongated Particle 5:1 Ratio	0
--	---

	Summary			
		Aver	rage	
	Component			
		Elongated	Non	
Particle Size	%	%	%	
-12.5mm + 9.5mm	15.9	0.6	99.4	
-9.5mm + 4.75mm	66.0	2.1	97.9	
Weighte	d Average =	1.8	98.2	

C.1.4 Granite Coarse Aggregate (North Carolina)

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Faiticle Size		g	G	g	g	%	%	%	%
	Trial 1	81.1	16.1	174.8	272	29.8	5.9	35.7	64.3
-12.5mm + 9.5mm	Trial 2	84.6	19.7	170.3	274.6	30.8	7.2	38.0	62.0
	Trial 3	133.4	52.9	357.7	544.0	24.5	9.7	34.2	65.8
	Trial 1	101.7	70.7	164.8	337.2	30.2	21.0	51.1	48.9
-9.5mm + 4.75mm	Trial 2	41.7	32.3	93.1	167.1	25.0	19.3	44.3	55.7
	Trial 3	28.2	15.0	40.4	83.6	33.7	17.9	51.7	48.3

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

Table C.16 Flat or Elongated Particle 2:1 Ratio

Summary								
	Component	Average						
	Component	Flat Elongated FOE						
Particle Size	%	% % %						
-12.5mm + 9.5mm	9.0	28.4	7.6	36.0	64.0			
-9.5mm + 4.75mm	67.0	29.6	19.4	49.0	51.0			
Weighte	Weighted Average =			47.5	52.5			

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
r article Size		g	G	g	g	%	%	%	%
	Trial 1	4.2	0.0	380.1	384.3	1.1	0.0	1.1	98.9
-12.5mm + 9.5mm	Trial 2	7.1	0.0	348.8	355.9	2.0	0.0	2.0	98.0
	Trial 3	1.6	0.0	333.6	335.2	0.5	0.0	0.5	99.5
	Trial 1	4.7	0.0	142.3	147.0	3.2	0.0	3.2	96.8
-9.5mm + 4.75mm	Trial 2	3.9	0.0	143.9	147.8	2.6	0.0	2.6	97.4
	Trial 3	6.5	0.0	129.4	135.9	4.8	0.0	4.8	95.2

Table C.17 Flat or Elongated Particle 3:1 Ratio

Summary								
	Component	Average						
	Component Flat Elongated FOE N							
Particle Size	%	% % %						
-12.5mm + 9.5mm	9.0	1.2	0.0	1.2	98.8			
-9.5mm + 4.75mm	75mm 67.0 3.5 0.0 3.5 96.5							
Weighted	Average =	3.3	0.0	3.3	96.7			

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	gg	g	%	%
	Trial 1	32.8	351.5	384.3	8.5	91.5
-12.5mm + 9.5mm	Trial 2	25.0	330.9	355.9	7.0	93.0
	Trial 3	18.3	316.9	335.2	5.5	94.5
	Trial 1	31.4	115.5	146.9	21.4	78.6
-9.5mm + 4.75mm	Trial 2	34.3	113.4	147.7	23.2	76.8
	Trial 3	28.4	107.5	135.9	20.9	79.1

Table C.18 Flat and Elongated Particle 3:1 Ratio

	Summary								
		Average							
	Component	Flat and							
		Elongated	Non						
Particle Size	%	%	%						
-12.5mm + 9.5mm	9.0	7.0	93.0						
-9.5mm + 4.75mm	67.0	21.8	78.2						
Weighte	d Average =	20.1	79.9						

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Fatucie Size		g	G	g	g	%	%	%	%
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-12.5mm + 9.5mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-9.5mm + 4.75mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL

Table C.19 Flat or Elongated Particle 5:1 Ratio

Summary								
	Component	Average						
	Component Flat Elongated FOE							
Particle Size	%	% % %						
-12.5mm + 9.5mm	9.0	0.0	0.0	0.0	100.0			
-9.5mm + 4.75mm	67.0	0.0	0.0	0.0	100.0			
Weighte	ed Average =	0.0	0.0	0.0	100.0			

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	g	g	%	%
	Trial 1	2.4	249.3	251.7	1.0	99.0
-12.5mm + 9.5mm	Trial 2	4.3	277.3	281.6	1.5	98.5
	Trial 3	3.3	234.6	237.9	1.4	98.6
	Trial 1	3.0	179.7	182.7	1.6	98.4
-9.5mm + 4.75mm	Trial 2	4.8	138.5	143.3	3.3	96.7
	Trial 3	4.9	132.5	137.4	3.6	96.4

Table C.20 Flat and Elongated Particle 5:1 Ratio

	Summary							
		Aver	age					
	Component	Flat and						
		Elongated	Non					
Particle Size	%	%	%					
-12.5mm + 9.5mm	9.0	1.3	98.7					
-9.5mm + 4.75mm	2.9	97.1						
Weighted	Weighted Average =							

C.1.5 Traprock #78 Coarse Aggregate (Virginia)

Doutiala Siza		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Particle Size		g	G	g	g	%	%	%	%
	Trial 1	81.6	15.5	728.0	825.1	9.9	1.9	11.8	88.2
-19mm + 12.5mm	Trial 2	62.1	14.1	694.4	770.6	8.1	1.8	9.9	90.1
	Trial 3	14.6	5.0	343.6	363.2	4.0	1.4	5.4	94.6
	Trial 1	138.0	95.2	541.6	774.8	17.8	12.3	30.1	69.9
-12.5mm + 9.5mm	Trial 2	123.5	94.7	574.1	792.3	15.6	12.0	27.5	72.5
	Trial 3	32.0	8.5	140.8	181.3	17.7	4.7	22.3	77.7
	Trial 1	108.2	67.3	194	369.5	29.3	18.2	47.5	52.5
-9.5mm + 4.75mm	Trial 2	78.5	69.8	172.3	320.6	24.5	21.8	46.3	53.7
	Trial 3	41.0	24.3	110.6	175.9	23.3	13.8	37.1	62.9

Validation of Performance-Related Test of Aggregates for Use in Hot-Mix Asphalt Pavements: Appendixes A through F

Table C.21 Flat or Elongated Particle 2:1 Ratio

	_	Summary						
			Average					
	Component	onent Flat Elongated FOE Non						
Particle Size	%	%	%	%	%			
-19mm + 12.5mm	11.0	7.3	1.7	9.0	91.0			
-12.5mm + 9.5mm	29.0	17.0	9.6	26.7	73.3			
-9.5mm + 4.75mm	53.0	25.7	17.9	43.6	56.4			
Weighte	ed Average =	20.8	13.4	34.2	65.8			

Dorticle Size	Particle Size	Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Faiticle Size		g	G	g	g	%	%	%	%
	Trial 1	0.0	0.0	352.1	352.1	0.0	0.0	0.0	100.0
-19mm + 12.5mm	Trial 2	0.0	0.0	331.0	331.0	0.0	0.0	0.0	100.0
	Trial 3	0.0	0.0	361.9	361.9	0.0	0.0	0.0	100.0
	Trial 1	5.6	4.2	240.3	250.1	2.2	1.7	3.9	96.1
-12.5mm + 9.5mm	Trial 2	4.3	0.0	216.0	220.3	2.0	0.0	2.0	98.0
	Trial 3	2.3	0.0	207.9	210.2	1.1	0.0	1.1	98.9
	Trial 1	9.0	3.1	118.3	130.4	6.9	2.4	9.3	90.7
-9.5mm + 4.75mm	Trial 2	7.5	1.3	118.5	127.3	5.9	1.0	6.9	93.1
	Trial 3	12.6	1.5	122.2	136.3	9.2	1.1	10.3	89.7

Table C.22 Fl	at or Elongated	Particle 3:1 Ratio
	at of Erongates	

Summary							
	Component		Ave	rage			
	Component	Flat	Elongated	FOE	Non		
Particle Size	%	%	%	%	%		
-19mm + 12.5mm	11	0.0	0.0	0.0	100.0		
-12.5mm + 9.5mm	29.0	1.8	0.6	2.3	97.7		
-9.5mm + 4.75mm	53.0	7.3	1.5	8.8	91.2		
Weighte	4.7	1.0	5.8	94.2			

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	G	g	%	%
	Trial 1	16.3	335.8	352.1	4.6	95.4
-19mm + 12.5mm	Trial 2	2.5	328.5	331.0	0.8	99.2
	Trial 3	13.3	348.6	361.9	3.7	96.3
	Trial 1	29.0	221.1	250.1	11.6	88.4
-12.5mm + 9.5mm	Trial 2	25.8	194.5	220.3	11.7	88.3
	Trial 3	8.0	202.2	210.2	3.8	96.2
	Trial 1	29.0	101.4	130.4	22.2	77.8
-9.5mm + 4.75mm	Trial 2	36.3	91.0	127.3	28.5	71.5
	Trial 3	41.4	94.9	136.3	30.4	69.6

Table C.23 Flat and Elongated Particle 3:1 Ratio

	Summary							
		Average						
	Component	Flat and Elongated	Non					
Particle Size	%	%	%					
-19mm + 12.5mm	11.0	3.0	97.0					
-12.5mm + 9.5mm	29.0	9.0	91.0					
-9.5mm + 4.75mm	53.0	27.0	73.0					
Weighte	d Average =	18.6	81.4					

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Faiticle Size		g	G	g	g	%	%	%	%
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-19mm + 12.5mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-12.5mm + 9.5mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-9.5mm + 4.75mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL

Table C.24 Flat or Elongated Particle 5:1 Ratio	Table C.24	Flat or	[•] Elongated	Particle 5:	1 Ratio
---	------------	---------	------------------------	-------------	---------

Summary							
	Component		Ave	rage			
	Component	Flat	Elongated	FOE	Non		
Particle Size	%	%	%	%	%		
-19mm + 12.5mm	11	0.0	0.0	0.0	100.0		
-12.5mm + 9.5mm	29.0	0.0	0.0	0.0	100.0		
-9.5mm + 4.75mm	53.0	0.0	0.0	0.0	100.0		
Weighte	0.0	0.0	0.0	100.0			

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	G	g	%	%
	Trial 1	6.4	559.2	565.6	1.1	98.9
-19mm + 12.5mm	Trial 2	0.0	573.2	573.2	0.0	100.0
	Trial 3	0.0	610.7	610.7	0.0	100.0
	Trial 1	7.2	365.4	372.6	1.9	98.1
-12.5mm + 9.5mm	Trial 2	2.2	362.7	364.9	0.6	99.4
	Trial 3	5.4	388.4	393.8	1.4	98.6
	Trial 1	5.0	194.3	199.3	2.5	97.5
-9.5mm + 4.75mm	Trial 2	6.0	176.9	182.9	3.3	96.7
	Trial 3	6.0	149.9	155.9	3.8	96.2

Table C.25 Flat and Elongated Particle 5:1 Ratio

Summary						
		Aver	age			
	Component	Flat and Elongated	Non			
Particle Size	%	%	%			
-19mm + 12.5mm	11.0	0.4	99.6			
-12.5mm + 9.5mm	29.0	1.3	98.7			
-9.5mm + 4.75mm	53.0	3.2	96.8			
Weighte	d Average =	2.3	97.7			

C.1.6 Traprock #88 Coarse Aggregate (Virginia)

Destigle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Particle Size		g	G	g	g	%	%	%	%
	Trial 1	17.3	0	130	147.3	11.7	0.0	11.7	88.3
-12.5mm + 9.5mm	Trial 2	14.6	0	131	145.6	10.0	0.0	10.0	90.0
	Trial 3	16.8	6.4	110.9	134.1	12.5	4.8	17.3	82.7
	Trial 1	23.6	14	68	105.6	22.3	13.3	35.6	64.4
-9.5mm + 4.75mm	Trial 2	24.5	14.0	60.0	98.5	24.9	14.2	39.1	60.9
	Trial 3	35.2	8.4	53	96.6	36.4	8.7	45.1	54.9

Table C.26 Flat or Elongated Particle 2:1 Ratio

Summary							
	Component	Average					
	Component	Flat Elongated FOE Non					
Particle Size	%	%	%	%	%		
-12.5mm + 9.5mm	14.6	11.4	1.6	13.0	87.0		
-9.5mm + 4.75mm	74.2	27.9 12.1 39.9 60.1					
Weighte	d Average =	25.2	10.3	35.5	64.5		

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
ratucie Size		g	G	g	g	%	%	%	%
	Trial 1	3.0	0.0	322.3	325.3	0.9	0.0	0.9	99.1
-12.5mm + 9.5mm	Trial 2	0.0	0.0	253.9	253.9	0.0	0.0	0.0	100.0
	Trial 3	2.5	0.0	305.3	307.8	0.8	0.0	0.8	99.2
	Trial 1	2.6	0.0	79.3	81.9	3.2	0.0	3.2	96.8
-9.5mm + 4.75mm	Trial 2	2.1	0.0	88.9	91.0	2.3	0.0	2.3	97.7
	Trial 3	2.3	0.0	153.3	155.6	1.5	0.0	1.5	98.5

Table C.27 Flat or Elongated Particle 3:1 Ratio

Summary						
			Ave	rage		
	Component	Flat	Elongated	FOE	Non	
Particle Size	%	%	%	%	%	
-12.5mm + 9.5mm	14.6	0.6	0.0	0.6	99.4	
-9.5mm + 4.75mm 74.2 2.3 0.0 2.3 97.7						
Weighte	d Average =	2.0	0.0	2.0	98.0	

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	G	g	%	%
	Trial 1	6.0	316.3	322.3	1.9	98.1
-12.5mm + 9.5mm	Trial 2	7.5	246.4	253.9	3.0	97.0
	Trial 3	5.2	302.6	307.8	1.7	98.3
	Trial 1	9.8	72.2	82.0	12.0	88.0
-9.5mm + 4.75mm	Trial 2	11.9	79.2	91.1	13.1	86.9
	Trial 3	23.7	132	155.7	15.2	84.8

Table C.28 Flat and Elongated Particle 3:1 Ratio

	Summary					
		Aver	age			
	Component	Flat and Elongated	Non			
Particle Size	%	%	%			
-12.5mm + 9.5mm	14.6	2.2	97.8			
-9.5mm + 4.75mm	74.2	13.4	86.6			
Weighte	Weighted Average = 11.6					

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
ratucie Size		g	G	gg	g	%	%	%	%
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-12.5mm + 9.5mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-9.5mm + 4.75mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL

Table C.29 Flat or Elongated Par	ticle 5:1 Ratio
----------------------------------	-----------------

Summary							
	Component	Average					
	Component	Component Flat Elongated FOE Non					
Particle Size	%	%	%	%	%		
-12.5mm + 9.5mm	14.6	0.0	0.0	0.0	100.0		
-9.5mm + 4.75mm 74.2 0.0 0.0 0.0 100.0							
Weighte	d Average =	0.0	0.0	0.0	100.0		

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	G	g	%	%
	Trial 1	8.0	265.5	273.5	2.9	97.1
-12.5mm + 9.5mm	Trial 2	1.3	222.7	224.0	0.6	99.4
	Trial 3	2.1	249.0	251.1	0.8	99.2
	Trial 1	3.8	170.6	174.4	2.2	97.8
-9.5mm + 4.75mm	Trial 2	4.7	164.4	169.1	2.8	97.2
	Trial 3	7.7	182.7	190.4	4.0	96.0

Table C.30 Flat and Elongated Particle 5:1 Ratio

	Summary						
	Component	Flat and					
		Elongated	Non				
Particle Size	%	%	%				
-12.5mm + 9.5mm	14.6	1.4	98.6				
-9.5mm + 4.75mm	74.2	3.0	97.0				
Weighte	Weighted Average =						

C.2 FLAT OR ELONGATED PARTICLE TEST (ASTM D4791) – HMA PLANT STOCKPILE SOURCES

C.2.1 Dolomite Coarse Aggregate (Indiana)

Particle Size	Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non	
Falticle Size		Ъ	G	g	g	%	%	%	%
	Trial 1	55.0	22.4	223.7	301.1	18.3	7.4	25.7	74.3
-12.5mm + 9.5mm	Trial 2	51.9	18.2	239.8	309.9	16.7	5.9	22.6	77.4
	Trial 3	73.0	15.9	127.1	360.5	20.2	4.4	24.7	35.3
	Trial 1	74.4	63.6	128.1	266.1	28.0	23.9	51.9	48.1
-9.5mm + 4.75mm	Trial 2	71.7	56.5	54.0	265	27.1	21.3	48.4	20.4
	Trial 3	80.8	49.6	45.1	278.2	29.0	17.8	46.9	16.2

Summary									
	Component		Ave	rage					
	Component	Flat	Elongated	FOE	Non				
Particle Size	%	% % %							
-12.5mm + 9.5mm	12.4	18.4	5.9	24.3	62.3				
-9.5mm + 4.75mm	59.9	28.0 21.0 49.0 28							
Weighte	ed Average =	26.4	18.4	44.8	34.1				

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Faiticle Size		g	g	g	g	%	%	%	%
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-12.5mm + 9.5mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	2.5	0.0	275.8	278.3	0.9	0.0	0.9	99.1
-9.5mm + 4.75mm	Trial 2	2.1	0.0	263.0	265.1	0.8	0.0	0.8	99.2
	Trial 3	2.2	0.0	263.4	265.6	0.8	0.0	0.8	99.2

Table C.32 Flat or Elongated Particle 5:1 Ratio

Summary									
	Component		Ave	rage					
	Component	FOE	Non						
Particle Size	%	% % %							
-12.5mm + 9.5mm	12.4	0.0	0.0	0.0	100.0				
-9.5mm + 4.75mm	59.9	0.9 0.8 0.0 0.8 99.2							
Weight	ed Average =	0.7	0.0	0.7	99.3				

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	g	g	%	%
	Trial 1	6.1	294.9	301	2.0	98.0
-12.5mm + 9.5mm	Trial 2	5.2	304.5	309.7	1.7	98.3
	Trial 3	5.0	355.7	360.7	1.4	98.6
	Trial 1	17.8	260.3	278.1	6.4	93.6
-9.5mm + 4.75mm	Trial 2	19.7	245.4	265.1	7.4	92.6
	Trial 3	13.0	252.9	265.9	4.9	95.1

Table C.33 Flat and Elongated Particle 5:1 F	Ratio
--	-------

	Summary								
		Aver	age						
	Component	Flat and Elongated	Non						
Particle Size	%	%	%						
-12.5mm + 9.5mm	12.4	1.7	98.3						
-9.5mm + 4.75mm	59.9	6.2	93.8						
Weighted	l Average =	5.5	94.5						

C.2.2 Limestone Coarse Aggregate (Indiana)

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Particle Size		g	g	g	g	%	%	%	%
	Trial 1	40.1	5.7	144.3	190.1	21.1	3.0	24.1	75.9
-12.5mm + 9.5mm	Trial 2	48.6	0.0	142.3	190.9	25.5	0.0	25.5	74.5
	Trial 3	27.0	12.6	104.5	144.1	18.7	8.7	27.5	72.5
	Trial 1	38.1	21.6	51.1	110.8	34.4	19.5	53.9	46.1
-9.5mm + 4.75mm	Trial 2	29.0	26.3	54.0	118.4	24.5	22.2	46.7	45.6
	Trial 3	26.4	16.0	53.1	95.5	27.6	16.8	44.4	55.6

Table C.34 Flat or Elongated Particle 2:1 Ratio

Summary									
	Component	Component Average							
	Component	Flat	FOE	Non					
Particle Size	%	% % %							
-12.5mm + 9.5mm	10.0	21.8	3.9	25.7	74.3				
-9.5mm + 4.75mm	56.8	28.8 19.5 48.3 49.1							
Weighte	ed Average =	27.8	17.2	44.9	52.9				

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Farticle Size		g	g	g	g	%	%	%	%
	Trial 1	0.0	0	ALL	ALL	0.0	0.0	0.0	ALL
-12.5mm + 9.5mm	Trial 2	0.0	0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0	0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	2.3	0	108.5	110.8	2.1	0.0	2.1	97.9
-9.5mm + 4.75mm	Trial 2	1.1	0	117.3	118.4	0.9	0.0	0.9	99.1
	Trial 3	1.7	0	115.5	117.2	1.5	0.0	1.5	98.5

Table C.35 Flat or Elongated Particle 5:1 Ratio

Summary									
	Component		Ave	rage					
	Component	FOE	Non						
Particle Size	%	% % %							
-12.5mm + 9.5mm	10.0	0.0	0.0	0.0	100.0				
-9.5mm + 4.75mm	56.8 1.5 0.0 1.5 98.5								
Weighte	ed Average =	1.3	0.0	1.3	98.7				

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	g	g	%	%
	Trial 1	3.4	186.5	189.9	1.8	98.2
-12.5mm + 9.5mm	Trial 2	4.1	186.2	190.3	2.2	97.8
	Trial 3	4.0	140.1	144.1	2.8	97.2
	Trial 1	9.6	100.8	110.4	8.7	91.3
-9.5mm + 4.75mm	Trial 2	12.0	106.6	118.6	10.1	89.9
	Trial 3	6.4	89.1	95.5	6.7	93.3

Table C.36 Flat and Elongated Particle 5:1 Ratio

	Summary					
		Aver	age			
	Component	Flat and Elongated	Non			
Particle Size	%	%	%			
-12.5mm + 9.5mm	10.0	2.3	98.0			
-9.5mm + 4.75mm	56.8	8.5	90.6			
Weighte	Weighted Average =					

C.2.3 Gravel Coarse Aggregate (Indiana)

Copyright National Academy of Sciences. All rights reserved.

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Faiticle Size		g	g	g	g	%	%	%	%
	Trial 1	27.3	0.0	148.8	176.1	15.5	0.0	15.5	84.5
-12.5mm + 9.5mm	Trial 2	26.3	0.0	133.8	160.1	16.4	0.0	16.4	83.6
	Trial 3	25.9	0.0	115.6	141.5	18.3	0.0	18.3	81.7
	Trial 1	37.7	21.6	100.7	160	23.6	13.5	37.1	62.9
-9.5mm + 4.75mm	Trial 2	38.4	22.3	54.0	162.4	23.6	13.7	37.4	33.3
	Trial 3	45.0	34.0	169.0	248.0	18.1	13.7	31.9	68.1

Table C.37 Flat or Elongated Particle 2:1 Ratio

	Summary									
	Component	Component Average								
	Component	Flat	Elongated	FOE	Non					
Particle Size	%	%	%	%	%					
-12.5mm + 9.5mm	5.7	16.7	0.0	16.7	83.3					
-9.5mm + 4.75mm	84.5	21.8	13.6	35.4	54.8					
Weight	ed Average =	21.8	13.6	35.4	54.8					

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Faiticle Size		g	g	g	g	%	%	%	%
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-12.5mm + 9.5mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-9.5mm + 4.75mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL

Table C.38 Flat or Elongated Particle 5:1 Ratio

	S	Summary			
	Component		Ave	rage	
	Component	Flat	Elongated	FOE	Non
Particle Size	%	%	%	%	%
-12.5mm + 9.5mm	5.7	0.0	0.0	0.0	100.0
-9.5mm + 4.75mm	84.5	0.0	0.0	0.0	100.0
Weight	0.0	0.0	0.0	100.0	

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	g	g	%	%
	Trial 1	1.4	174.7	176.1	0.8	99.2
-12.5mm + 9.5mm	Trial 2	2.9	157.2	160.1	1.8	98.2
	Trial 3	0.0	141.5	141.5	0.0	100.0
	Trial 1	0.3	159.7	160.0	0.2	99.8
-9.5mm + 4.75mm	Trial 2	2.9	160.1	163.0	1.8	98.2
	Trial 3	1.6	193.3	194.9	0.8	99.2

Table C.39 Flat and Elongated Particle 5:1 Ratio

	Summary		
		Aver	age
	Component	Flat and Elongated	Non
Particle Size	%	%	%
-12.5mm + 9.5mm	5.7	0.9	98.7
-9.5mm + 4.75mm	84.5	0.9	99.0
Weighted	l Average =	0.9	99.0

C.2.4 Granite Coarse Aggregate (North Carolina)

Particle Size	Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non	
Faiticle Size		g	g	g	gg	%	%	%	%
	Trial 1	51.9	17.1	157.8	226.8	22.9	7.5	30.4	69.6
-12.5mm + 9.5mm	Trial 2	46.5	15.0	155.8	217.3	21.4	6.9	28.3	71.7
	Trial 3	47.3	23.7	129.1	200.1	23.6	11.8	35.5	64.5
	Trial 1	72.5	60.0	74.9	207.4	35.0	28.9	63.9	36.1
-9.5mm + 4.75mm	Trial 2	39.3	26.2	39.8	105.3	37.3	24.9	62.2	37.8
	Trial 3	35.4	23.9	44.1	103.4	34.2	23.1	57.4	42.6

Table C.40 Flat or Elongated Particle 2:1 Ratio

	Summary									
	Component	Average								
	Component	Flat	Elongated	FOE	Non					
Particle Size	%	%	%	%	%					
-12.5mm + 9.5mm	16.2	22.6	8.8	31.4	68.6					
-9.5mm + 4.75mm	64.7	35.5	25.6	61.1	38.9					
Weighte	32.9	22.3	55.2	44.8						

Particle Size	Particle Size	Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
Faiticle Size		g	g	gg	g	%	%	%	%
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-12.5mm + 9.5mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-9.5mm + 4.75mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL

Table C.41 Flat or Elongated Particle 5:1 Ratio

	Summary									
	Component	Average								
	Component	Flat	Elongated	FOE	Non					
Particle Size	%	%	%	%	%					
-12.5mm + 9.5mm	16.2	0.0	0.0	0.0	100.0					
-9.5mm + 4.75mm	64.7	0.0	0.0	0.0	100.0					
Weighte	0.0	0.0	0.0	100.0						

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	gg	ъ	%	%
	Trial 1	3.8	222.9	226.7	1.7	98.3
-12.5mm + 9.5mm	Trial 2	6.0	211.3	217.3	2.8	97.2
	Trial 3	0.0	200.1	200.1	0.0	100.0
	Trial 1	16.1	191.2	207.3	7.8	92.2
-9.5mm + 4.75mm	Trial 2	9.1	96.0	105.1	8.7	91.3
	Trial 3	7.0	96.4	103.4	6.8	93.2

Table C.42 Flat and Elongated Particle 5:1 Ratio	С
--	---

Summary							
		Average					
	Component	Flat and Elongated	Non				
Particle Size	%	%	%				
-12.5mm + 9.5mm	16.2	1.5	98.5				
-9.5mm + 4.75mm	64.7	7.8	92.2				
Weighte	Weighted Average =						

C.2.5 Traprock #88 Coarse Aggregate (Virginia)

Particle Size	Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non	
Faiticle Size		g	G	g	g	%	%	%	%
-12.5mm + 9.5mm	Trial 1	17.3	0.0	130	147.3	11.7	0.0	11.7	88.3
	Trial 2	14.6	0.0	131	145.6	10.0	0.0	10.0	90.0
	Trial 3	16.8	6.4	110.9	134.1	12.5	4.8	17.3	82.7
-9.5mm + 4.75mm	Trial 1	23.6	14.0	68	105.6	22.3	13.3	35.6	64.4
	Trial 2	24.5	14.0	60.0	98.5	24.9	14.2	39.1	60.9
	Trial 3	35.2	8.4	53	96.6	36.4	8.7	45.1	54.9

Table C.43 Flat or Elongated Particle 2:1 Ratio

Summary							
	Component	Average					
	Component	Flat	Elongated	FOE	Non		
Particle Size	%	%	%	%	%		
-12.5mm + 9.5mm	19.0	11.4	1.6	13.0	87.0		
-9.5mm + 4.75mm	70.8	27.9	12.1	39.9	60.1		
Weighted Average =		24.4	9.8	34.3	65.7		

Particle Size		Flat	Elongated	Non	Total	Flat	Elongated	FOE	Non
I alticle Size		g	G	g	g	%	%	%	%
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-12.5mm + 9.5mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 1	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
-9.5mm + 4.75mm	Trial 2	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL
	Trial 3	0.0	0.0	ALL	ALL	0.0	0.0	0.0	ALL

Table C.44 Flat or Elongated Particle 5:1 Ratio

Summary							
		Average					
	Component	Flat	Elongated	FOE	Non		
Particle Size	%	%	%	%	%		
-12.5mm + 9.5mm	19.0	0.0	0.0	0.0	100.0		
-9.5mm + 4.75mm	70.8	0.0	0.0	0.0	100.0		
Weighted Average =		0.0	0.0	0.0	100.0		

Particle Size		Flat and Elongated	Non	Total	Flat and Elongated	Non
		g	G	g	%	%
	Trial 1	0.0	ALL	ALL	0.0	ALL
-12.5mm + 9.5mm	Trial 2	0.0	ALL	ALL	0.0	ALL
	Trial 3	0.0	ALL	ALL	0.0	ALL
	Trial 1	5.2	100.5	105.7	4.9	95.1
-9.5mm + 4.75mm	Trial 2	3.6	94.4	98.0	3.7	96.3
	Trial 3	3.9	92.9	96.8	4.0	96.0

Table C.45 Flat and Elongated Particle 5:1 Ratio

Summary								
	Average							
	Component	Flat and Elongated	Non					
Particle Size	%	%	%					
-12.5mm + 9.5mm	19.0	0.0	100.0					
-9.5mm + 4.75mm	70.8	4.2	95.8					
Weighte	3.3	96.7						

C.3 UNCOMPACTED VOID CONTENT (AASHTO TP56) - LABORATORY MIX DESIGN SOURCE

C.3.1 Method A – AASHTO TP56

Table C.46 Coarse Aggregate

Aggregate Type	Dolomite	Limestone	Gravel	Granite	Traprock #78	Traprock #88
Dry Bulk Specific Gravity, g/cm3	2.734	2.550	2.598	2.674	2.897	2.910
Dry Weight 1, g	3951.4	3903.1	4429.2	4021.2	4403.2	4468.3
Dry Weight 2, g	3947.7	3898.6	4437.1	4052.4	4397.5	4476.5
Dry Weight 3, g	3934.2	3907.1	4447.5	4010.5	4394.9	4481.1
Average Dry Weight, g	3944.4	3902.9	4437.9	4036.8	4398.5	4475.3
UVA, %	51.2	48.2	42.2	48.9	48.6	48.0

Note: Volume of Container = 2954.6 cm^3

C.3.2 Method B – AASHTO TP56

Table C.47 Dolomite Coarse Aggregate

Data

Size Fraction	<u>-12.5</u> + 9	9.5 <u>mm</u>		<u>-9.</u>
	Dry Weight 1	4139.7	g	Dry Weight 1
	Dry Weight 2	4174.2	g	Dry Weight 2
	Dry Weight 3	4146.8	g	Dry Weight 3
	Average Dry Weight	4153.6	g	Average Dry W
Calculation				
Size Fraction	<u>-12.5 +9</u>	9.5mm		<u>-9.</u>
	U1 =	48.6		
	UVB =	49.8		

4.75mm	
3957.7	g
3973.3	g
3963.2	g
3964.7	g
	3973.3 3963.2

<u>-9.5mm +4</u>	.75mm
U2 =	50.9

Data

Size Fraction	<u>-12.5</u> + 9	9.5mm		<u>-9.5mm</u> +	4.75mm	
	Dry Weight 1	3863.7	g	Dry Weight 1	3740.1	g
	Dry Weight 2	3850.1	g	Dry Weight 2	3717.8	g
	Dry Weight 3	3875.3	g	Dry Weight 3	3738.5	g
	Average Dry Weight	3863.0	g	Average Dry Weight	3732.1	g
Calculation						
Size Fraction	<u>-12.5 +9</u>	9.5 <u>mm</u>		<u>-9.5mm +</u>	<u>4.75mm</u>	
	U1 =	48.8		U2 =	50.5	
	UVB =	49.6				

Table C.49 Gravel Coarse Aggregate

Data						
Size Fraction	<u>-12.5 + 9</u>	9.5mm		<u>-9.5mm +</u>	4.75mm	
	Dry Weight 1	4417.1	g	Dry Weight 1	4361.8	g
	Dry Weight 2	4371.0	g	Dry Weight 2	4382.4	g
	Dry Weight 3	4418.5	g	Dry Weight 3	4373.0	g
	Average Dry Weight	4402.2	g	Average Dry Weight	4372.4	g
Calculation						
Size Fraction	<u>-12.5 + 9</u>	9.5 <u>mm</u>		<u>-9.5mm</u> +	4.75mm	
	U1 =	42.7		U2 =	43.1	
	UVB =	42.9				

g g g g

Table C.50 Granite Coarse Aggregate

Data

Copyright National Academy of Sciences. All rights reserved.

Size Fraction	-12.5 +2	9.5mm		<u>-9.5mm</u> +	4.75mm	
	Dry Weight 1	4042.0	g	Dry Weight 1	3998.7	g
	Dry Weight 2	4101.0	g	Dry Weight 2	3972.4	g
	Dry Weight 3	4031.2	g	Dry Weight 3	3977.7	g
	Average Dry Weight	4058.1	g	Average Dry Weight	3982.9	g
Calculation						
Size Fraction	<u>-12.5 + 9</u>	9.5 <u>mm</u>		<u>-9.5mm</u> +	4.75mm	
	U1 =	48.7		U2 =	49.6	
	UVB =	49.1				

Table C.51 Traprock #78 Coarse Aggregate

Data									
Size Fraction	<u>-19.0+12.5</u>	<u>mm</u>		<u>-12.5</u> +9.5r	<u>nm</u>		<u>-9.5mm</u> +	<u>4.75mm</u>	
	Dry Weight 1	4523.6	g	Dry Weight 1	4431.2	g	Dry Weight 1	4287.9	g
	Dry Weight 2	4531.6	g	Dry Weight 2	4424.2	g	Dry Weight 2	4285.5	g
	Dry Weight 3	4580.1	g	Dry Weight 3	4393.7	g	Dry Weight 3 Average Dry	4317.4	g
	Average Dry Weight	4545.1	g	Average Dry Weight	4416.4	g	Weight	4296.9	g
Calculation									
Size Fraction	<u>-12.5 +9.5</u>	<u>nm</u>		<u>-12.5 +9.5</u>	<u>nm</u>		<u>-9.5mm +</u>	<u>4.75mm</u>	
	U1 =	46.9		U1 =	48.4		U2 =	= 49.8	
	UVB =	48.4							

Table C.52 Traprock #88 Coarse Aggregate

Data

Size Fraction	<u>-12.5 +9</u>	9.5mm		<u>-9.5mm +</u>	4.75mm	
	Dry Weight 1	4432.2	g	Dry Weight 1	4350.5	g
	Dry Weight 2	4456.0	g	Dry Weight 2	4357.9	g
	Dry Weight 3	4459.0	g	Dry Weight 3	4361.7	g
	Average Dry Weight	4449.1	g	Average Dry Weight	4356.7	g
Calculation						
Size Fraction	<u>-12.5 +9</u>	9.5mm		<u>-9.5mm +</u>	<u>4.75mm</u>	
	U1=	48.3		U2=	49.4	
	UVB=	48.8				

C.4 MICRO-DEVAL TEST

Aggregate Type	Dolomite	Limestone	Gravel	Granite	Traprock #78	Traprock #88
Source	Indiana	Indiana	Indiana	North Carolina	Virginia	Virginia
Initial Dry Weight, g	1502.6	1501.2	1500.9	1500.8	1500.7	1500.1
Final Dry Weight, g	1397.5	1338.3	1368.9	1419.0	1423.7	1430.4
Weight Loss, g	105.1	162.9	132.0	81.8	77.0	69.7
Micro Deval Value	7.0	10.9	8.8	5.5	5.1	4.6

Table C.53 Micro-Deval Tests (AASHTO TP58) - Coarse Aggregates

Note: Average of 3 samples

Table C.54 Micro-Deval Tests (Ontario Test Method LS-619) - Fine Aggregates

Aggregate Type	Natural Sand A	Crushed Gravel Sand	Natural Sand B	Granite Sand	Dolomite Sand	Traprock #16	Dolomite Sand	Traprock #13
Source	Indiana	Indiana	Ohio	North Carolina	Indiana	Virginia	Indiana	Virginia
Initial Dry Weight, g	499.9	499.9	499.8	500.0	500.0	500.0	499.9	499.9
Final Dry Weight, g	449.8	415.1	397.6	446.9	471.1	439.4	409.3	427.4
Weight Loss, g	50.1	84.8	102.2	53.1	28.9	60.6	90.6	72.5
Micro Deval Value	10.0	17.0	20.4	10.6	5.8	12.1	18.1	14.5

C.5 LOS ANGELES ABRASION TEST (ASTM C96)

Aggregate	Gradation Type	Initial Dry Weight, g	Weight Retained on the 1.7 mm Sieve, g	Loss, %
Dolomite	С	5000.8	3850.6	23.0
Limestone	С	5000.3	3758.9	24.8
Gravel	С	5000.3	4060.1	18.8
Granite	С	5000.3	4057.1	18.9
Traprock #78	С	5000.5	4320.4	13.6
Traprock #88	С	4999.7	4285.6	14.3

C.6 MAGNESIUM SULFATE SOUNDNESS TEST (AASHTO T104) - 5 CYCLES

Table C.56 Dolomite Coarse Aggregate

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
19.0mm to 12.5mm					
12.5mm to 9.5mm	13.4	281.3	280.9	0.1	0.03
9.5mm to 4.75mm	61.9	320.2	315.8	1.4	1.13
Total =	75.3			Total =	1
19.0mm to 12.5mm					
12.5mm to 9.5mm	13.4	334.3	332.8	0.4	0.08
9.5mm to 4.75mm	61.9	300.3	299.2	0.4	0.30
Total =	75.3			Total =	0

19.0mm to 12.5mm					
12.5mm to 9.5mm	13.4	333.1	330.5	0.8	0.14
9.5mm to 4.75mm	61.9	302.8	300.5	0.8	0.62
Total =	75.3			Total =	1

Average Loss = $\underline{1}$

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
19.0mm to 12.5mm					
12.5mm to 9.5mm	12.1	327.2	316.1	3.4	0.55
9.5mm to 4.75mm	63.2	320.3	299.1	6.6	5.56
Total =	75.3			Total =	6

19.0mm to 12.5mm					
12.5mm to 9.5mm	12.1	365.3	356.7	2.4	0.38
9.5mm to 4.75mm	63.2	329.2	309.6	6.0	5.00
Total =	75.3			Total =	5

19.0mm to 12.5mm					
12.5mm to 9.5mm	12.1	362.7	341.7	5.8	0.93
9.5mm to 4.75mm	63.2	315.6	291	7.8	6.54
Total =	75.3	•	•	Total =	7

Average Loss = $\underline{\mathbf{6}}$

Copyright National Academy of Sciences. All rights reserved	Sieve Size	Grading of Original Fractions Sample
nt Na	19.0mm to 12.5mm	
ation	12.5mm to 9.5mm	15.9
al A	9.5mm to 4.75mm	66.0
cadem	Total =	81.9
y of	19.0mm to 12.5mm	
Scie	12.5mm to 9.5mm	15.9
nce	9.5mm to 4.75mm	66.0
s. All ri	Total =	81.9
ghts	19.0mm to 12.5mm	
rese	12.5mm to 9.5mm	15.9
eve	9.5mm to 4.75mm	66.0
<u>a</u>	Total –	Q1 O

Table C.58 Gravel Coarse Aggregate

Total =	81.9
---------	------

Total =

Percentage

Passing

Designated

Sieve After

Test

7.2

5.6

5.9

8.3

Total =

Weighted

Percentage

Loss

1.40

4.51

6

1.14

6.71

8

19.0mm to 12.5mm					
12.5mm to 9.5mm	15.9	336.1	319.2	5.0	0.98
9.5mm to 4.75mm	66.0	300.4	282.1	6.1	4.91
Total =	81.9			Total =	6

Mass of

Test

Fraction

Before Test

324.8

300.4

321

297.7

Mass of

Test

Fraction

After Test

301.3

283.6

302.2

272.9

Average Loss = <u>7</u>

Copyright National Academy of Sciences. All rights reserved.	
All rights reserved.	

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
19.0mm to 12.5mm					
12.5mm to 9.5mm	9.0	323.5	323.3	0.1	0.01
9.5mm to 4.75mm	67.0	329.8	327.5	0.7	0.61
Total =	76.0			Total =	1
19.0mm to 12.5mm					
12.5mm to 9.5mm	9.0	353.4	353.1	0.1	0.01
9.5mm to 4.75mm	67.0	378.7	378.1	0.2	0.14
Total =	76.0			Total =	0
19.0mm to 12.5mm					
12.5mm to 9.5mm	9.0	349	348.5	0.1	0.02
9.5mm to 4.75mm	67.0	286.7	285.5	0.4	0.37

 Table C.59 Granite Coarse Aggregate

Total =

76.0

Average Loss = $\underline{\mathbf{0}}$

Total =

0

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
19.0mm to 12.5mm					
12.5mm to 9.5mm	29.0	354.2	350.8	1.0	0.34
9.5mm to 4.75mm	53.0	398.5	394.3	1.1	0.68
Total =	82.0			Total =	1

19.0mm to 12.5mm					
12.5mm to 9.5mm	29.0	341.3	336.5	1.4	0.50
9.5mm to 4.75mm	53.0	384.8	381.2	0.9	0.60
Total =	82.0			Total =	1

19.0mm to 12.5mm					
12.5mm to 9.5mm	29.0	352.3	350.7	0.5	0.16
9.5mm to 4.75mm	53.0	384.1	380.4	1.0	0.62
Total =	82.0			Total =	1

Average Loss= <u>1</u>

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
19.0mm to 12.5mm					
12.5mm to 9.5mm	14.6	338.7	335.5	0.9	0.16
9.5mm to 4.75mm	74.2	319.3	315.8	1.1	0.92
Total =	88.8			Total =	1

Total :	= 8
---------	-----

al	=		

19.0mm to 12.5mm					
12.5mm to 9.5mm	14.6	336.2	333.2	0.9	0.15
9.5mm to 4.75mm	74.2	318.8	314.8	1.3	1.05
— 1		•		— 1	

Total = 88.8 Total = 1

Average Loss = <u>1</u>

C.7 UNCOMPACTED VOID CONTENT OF FINE AGGREGATES

Table C.62 Method A – ASTM C1252

Aggregate Type	Natural Sand A	Crushed Gravel Sand	Natural Sand B	Granite Sand	Dolomite Sand	Dolomite Sand	Traprock #16	Traprock #13
Dry Bulk Specific Gravity, g/cm3	2.585	2.660	2.586	2.639	2.665	2.634	2.911	2.892
Dry Weight 1, g	153.5	142.9	149.4	133.7	146.1	138.4	149.1	145.7
Dry Weight 2, g	153.8	142.6	149.7	133.3	145.5	139.1	147.7	146.3
Dry Weight 3, g	153.3	142.3	149.2	133.8	145.5	140.0	148.1	146.0
Average Dry Weight, g	153.5	142.6	149.4	133.6	145.7	139.2	148.3	146.0
UVA, %	40.2	46.1	41.9	49.1	45.0	46.8	48.8	49.2

Table C.63 Natural Sand A (Method B – ASTM C1252)

Data

Size Fraction	<u>-2.36mm</u> +	1.18mm		<u>-1.18mm + 0.60mm</u>			<u>-0.60mm + 0.30mm</u>		
	Trial 1	150.1	g	Trial 1	144.6	g	Trial 1	143.3	g
	Trial 2	151.0	g	Trial 2	144.8	g	Trial 2	143.3	g
	Trial 3	150.3	g	Trial 3	144.8	g	Trial 3	143.4	g
	Average	150.5	g	Average	144.7	g	Average	143.3	g
Calculation									
	U1=	41.4		U2=	43.7		U3=	44.2	
	UVB =	43.1							

Table C.64 Natural Sand A (VTM5)

Data									
Size Fraction	<u>-2.36mm</u> +	1.00mm		<u>-1.00mm</u> +	0.60mm		<u>-0.60mm</u> +	- 0.30mm	
	Trial 1	413.5	g	Trial 1	406.2	g	Trial 1	405.2	g
	Trial 2	413.3	g	Trial 2	406.1	g	Trial 2	405.1	g
	Trial 3	412.7	g	Trial 3	405.1	g	Trial 3	405.1	g
	Average	413.2	g	Average	405.8	g	Average	405.1	g
Calculation									
	U1=	43.3		U2=	44.3		U3=	44.4	
	UVB =	44.0							

Table C.65 Crushed Gravel Sand (Method B – ASTM C1252)

Data									
Size Fraction	<u>-2.36mm</u> +	1.18mm		<u>-1.18mm</u> +	0.60mm		<u>-0.60mm + 0.30mm</u>		
	Trial 1	136.2	g	Trial 1	129.2	g	Trial 1	127.5	g
	Trial 2	136.5	g	Trial 2	129.3	g	Trial 2	127.4	g
	Trial 3	136.5	g	Trial 3	129.2	g	Trial 3	127.5	g
	Average	136.4	g	Average	129.2	g	Average	127.5	g
Calculation									
	U1=	48.4		U2=	51.1		U3=	51.8	
	UVB =	50.4							

Table C.66 Crushed Gravel Sand (VTM5)

Data									
Size Fraction	<u>-2.36mm</u> +	<u>-2.36mm + 1.00mm</u>			0.60mm		<u>-0.60mm + 0.30mm</u>		
	Trial 1	368.6	g	Trial 1	360.8	g	Trial 1	358.4	g
	Trial 2	369.3	g	Trial 2	360.2	g	Trial 2	358.3	g
	Trial 3	369.6	g	Trial 3	360.8	g	Trial 3	360.1	g
	Average	369.2	g	Average	360.6	g	Average	358.9	g
Calculation									
	U1=	50.8		U2=	51.9		U3=	52.1	
	UVB =	51.6							

Table C.67 Natural Sand B (Method B – ASTM C1252)

Data									
Size Fraction	<u>-2.36mm</u> +	1.18mm		<u>-1.18mm</u> +	0.60mm		<u>-0.60mm + 0.30mm</u>		
	Trial 1	144.6	g	Trial 1	137.9	g	Trial 1	133.7	g
	Trial 2	144.5	g	Trial 2	137.8	g	Trial 2	133.7	g
	Trial 3	143.8	g	Trial 3	137.6	g	Trial 3	133.6	g
	Average	144.3	g	Average	137.8	g	Average	133.7	g
Calculation									
	U1=	44.1		U2=	46.7		U3=	48.3	
	UVB =	46.4							

Table C.68 Natural Sand B (VTM5)

Data									
Size Fraction	<u>-2.36mm</u> +	1.00mm		<u>-1.00mm</u> +	0.60mm		<u>-0.60mm</u> +	0.30mm	
	Trial 1	395.3	g	Trial 1	386.1	g	Trial 1	377.9	g
	Trial 2	396.4	g	Trial 2	384.7	g	Trial 2	376.2	g
	Trial 3	394.0	g	Trial 3	386.5	g	Trial 3	375.8	g
	Average	395.2	g	Average	385.8	g	Average	376.6	g
Calculation									
	U1=	46.1		U2=	47.3		U3=	48.6	
	UVB =	47.3							

Table C.69 Granite Sand (Method B – ASTM C1252)

Data									
Size Fraction	<u>-2.36mm</u> +	1.18mm		<u>-1.18mm</u> +	0.60mm		<u>-0.60mm</u> +	0.30mm	
	Trial 1	127.9	g	Trial 1	122.2	g	Trial 1	120.0	g
	Trial 2	128.6	g	Trial 2	122.5	g	Trial 2	119.8	g
	Trial 3	127.5	g	Trial 3	122.0	g	Trial 3	120.0	g
	Average	128.0	g	Average	122.2	g	Average	119.9	g
Calculation									
	U1=	51.2		U2=	53.4		U3=	54.3	
	UVB =	53.0							

Table C.70 Granite Sand (VTM5)

Data									
Size Fraction	<u>-2.36mm</u> +	-2.36mm + 1.00mm			0.60mm		<u>-0.60mm</u> +	- 0.30mm	
	Trial 1	342.2	g	Trial 1	338.4	g	Trial 1	337.7	g
	Trial 2	342.4	g	Trial 2	336.2	g	Trial 2	337.9	g
	Trial 3	342.2	g	Trial 3	335.9	g	Trial 3	337.8	g
	Average	342.3	g	Average	336.8	g	Average	337.8	g
Calculation									
	U1=	54.0		U2=	54.7		U3=	54.6	
	UVB =	54.4							

Table C.71 Dolomite Sand (Method B – ASTM C1252)

Data									
Size Fraction	<u>-2.36mm</u> +	1.18mm		<u>-1.18mm</u> +	0.60mm		<u>-0.60mm + 0.30mm</u>		
	Trial 1	134.7	g	Trial 1	131.4	g	Trial 1	131.4	g
	Trial 2	134.8	g	Trial 2	131.4	g	Trial 2	131.5	g
	Trial 3	134.9	g	Trial 3	131.7	g	Trial 3	131.6	g
	Average	134.8	g	Average	131.5	g	Average	131.5	g
Calculation									
	U1=	49.1		U2=	50.4		U3=	50.4	
	UVB =	49.9							

Table C.72 Dolomite Sand (VTM5)

Data									
Size Fraction	<u>-2.36mm</u> +	<u>-2.36mm + 1.00mm</u>			0.60mm		<u>-0.60mm + 0.30mm</u>		
	Trial 1	365.6	g	Trial 1	363.9	g	Trial 1	370	g
	Trial 2	365.7	g	Trial 2	363.3	g	Trial 2	370.5	g
	Trial 3	365.5	g	Trial 3	362.2	g	Trial 3	370.6	g
	Average	365.6	g	Average	363.1	g	Average	370.4	g
Calculation									
	U1=	51.3		U2=	51.7		U3=	50.7	
	UVB =	51.2							

Table C.73 Traprock #16 Sand (Method B – ASTM C1252)

Data									
Size Fraction	<u>-2.36mm</u> +	1.18mm		<u>-1.18mm</u> +	0.60mm		<u>-0.60mm</u> +	0.30mm	
	Trial 1	139.0	g	Trial 1	133.3	g	Trial 1	132.8	g
	Trial 2	138.8	g	Trial 2	133.6	g	Trial 2	132.7	g
	Trial 3	138.4	g	Trial 3	133	g	Trial 3	132.6	g
	Average	138.7	g	Average	133.3	g	Average	132.7	g
Calculation									
	U1=	52.3		U2=	54.1		U3=	54.3	
	UVB =	53.6							

Table C.74 Traprock #16 Sand (VTM5)

Data									
Size Fraction	<u>-2.36mm</u> +	1.00mm		<u>-1.00mm</u> +	0.60mm		<u>-0.60mm</u> +	0.30mm	
	Trial 1	370.8	g	Trial 1	369.1	g	Trial 1	375.1	g
	Trial 2	371.9	g	Trial 2	368.6	g	Trial 2	375.3	g
	Trial 3	372.5	g	Trial 3	368.3	g	Trial 3	375.4	g
	Average	371.7	g	Average	368.6	g	Average	375.3	g
Calculation									
	U1=	54.9		U2=	55.3		U3=	54.5	
	UVB =	54.9							

Table C.75 Dolomite Sand (Method B – ASTM C1252)

Data									
Size Fraction	<u>-2.36mm +</u>	<u>1.18mm</u>		<u>-1.18mm + 0.60mm</u>			<u>-0.60mm + 0.30mm</u>		
	Trial 1	131.6	g	Trial 1	127.5	g	Trial 1	126.3	g
	Trial 2	131.3	g	Trial 2	127.4	g	Trial 2	126.9	g
	Trial 3	131.6	g	Trial 3	127.8	g	Trial 3	126.6	g
	Average	131.5	g	Average	127.6	g	Average	126.6	g
Calculation									
	U1=	49.8		U2=	51.3		U3=	51.6	
	UVB =	50.9							

Table C.76 Dolomite Sand (VTM5)

Data									
Size Fraction	<u>-2.36mm</u> +	1.00mm		<u>-1.00mm</u> +	0.60mm		<u>-0.60mm</u> +	0.30mm	
	Trial 1	357.3	g	Trial 1	356.0	g	Trial 1	356.7	g
	Trial 2	357.9	g	Trial 2	355.5	g	Trial 2	357.1	g
	Trial 3	358.0	g	Trial 3	356.4	g	Trial 3	357.2	g
	Average	357.7	g	Average	356.0	g	Average	357.0	g
Calculation									
	U1=	51.8		U2=	52.1		U3=	51.9	
	UVB =	51.9							

Table C.77 Traprock #13 Sand (Method B – ASTM C1252)

Data									
Size Fraction	<u>-2.36mm</u> +	1.18mm		<u>-1.18mm</u> +	0.60mm		<u>-0.60mm</u> +	0.30mm	
	Trial 1	138.1	g	Trial 1	132.3	g	Trial 1	130.1	g
	Trial 2	138.1	g	Trial 2	132.3	g	Trial 2	130.2	g
	Trial 3	138.1	g	Trial 3	132.1	g	Trial 3	130.1	g
	Average	138.1	g	Average	132.2	g	Average	130.1	g
Calculation									
	U1=	52.0		U2=	54.0		U3=	54.7	
	UVB =	53.6							

Table C.78 Traprock #13 Sand (VTM5)

Data										
Size Fraction	<u>-2.36mm</u> +	<u> 2.36mm + 1.00mm</u>			<u>-1.00mm + 0.60mm</u>			-0.60mm + 0.30mm		
	Trial 1	367.8	g	Trial 1	364.6	g	Trial 1	366.6	g	
	Trial 2	368.0	g	Trial 2	364.8	g	Trial 2	366.1	g	
	Trial 3	367.7	g	Trial 3	363.9	g	Trial 3	365.8	g	
	Average	367.8	g	Average	364.4	g	Average	366.2	g	
Calculation										
	U1=	54.9		U2=	55.3		U3=	55.1		
	UVB =	55.1								

C.8 METHYLENE BLUE TEST (AASHTO TP57)

Table C.79 Laboratory Mix Design Source

	A	ggregate Type		Aggregate Type			
	N	latural Sand A		Crushed Gravel Sand			
	Initial Reading	Final Reading	MBV	Initial Reading	Final Reading	MBV	
Test 1	26.5	33.0	3.3	33.5	36.0	1.3	
Test 2	16.0	22.5	3.3	29.0	31.5	1.3	
Test 3	22.5	29.0	3.3	31.5	34.0	1.3	
	A	verage MBV =	3.3	A	verage MBV =	1.3	

	A	ggregate Type		Aggregate Type			
	(Granite Sand		Dolomite Sand			
	Initial Reading	Final Reading	MBV	Initial Reading	Final Reading	MBV	
Test 1	35.5	51.5	8.0	32.5	33.5	0.5	
Test 2	22.5	47.0	12.3	1.5	3.0	0.8	
Test 3	3.0	29.0	13.0	17	18.5	0.8	
	A	verage MBV =	11.1	A	verage MBV =	0.7	

Average MBV = 11.1

Average MBV = 0.7

	A	ggregate Type		A	ggregate Type		
	-	Traprock #16		Traprock #13			
	Initial Reading	Final Reading	MBV	Initial Reading	Final Reading	MBV	
Test 1	10.5	24.0	6.8	14.5	24.0	4.8	
Test 2	-	-	-	13.5	24.0	5.3	
Test 3	-	-	-	24	34.5	5.3	
	٨		~ ~	^		F 4	

Average MBV = 6.8 Average MBV = 5.1

	A	ggregate Type		Aggregate Type		
	N	latural Sand B		Dolomite Sand		
	Initial Reading	Final Reading	MBV	Initial Reading	Final Reading	MBV
Test 1	19.5	29.5	5.0	19	24.5	2.8
Test 2	3.0	12.5	4.8	24.5	30.0	2.8
Test 3	12.5	22.5	5.0	-	-	-
	Ą	verage MBV =	4.9	A	verage MBV =	2.8

Table C.80 HMA Plant Stockpiles

	A	ggregate Type		A	ggregate Type		
	N	latural Sand A		Crushed Gravel Sand			
	Initial	Final		Initial	Final		
	Reading	Reading	MBV	Reading	Reading	MBV	
Test 1	9.0	22.5	6.8	22.5	25.0	1.3	
Test 2	1.5	18.0	8.3	8.0	10.5	1.3	
Test 3	18.0	34.5	8.3	10.5	13.0	1.3	
	٨	Verege MDV	7.0	/	Vierona MDV	10	

Average MBV = 7.8 Average

e MBV = 1.3	3
-------------	---

	A	ggregate Type		A	ggregate Type		
	(Granite Sand		Dolomite Sand			
	Initial	Final		Initial	Final		
	Reading	Reading	MBV	Reading	Reading	MBV	
Test 1	12.5	24.0	5.8	0.5	6	2.8	
Test 2	24.0	36.0	6.0	6.0	11.5	2.8	
Test 3	35.0	46.5	5.8	11.5	17	2.8	
	A	verage MBV =	5.8	ŀ	verage MBV =	2.8	

	Aggregate Type			A	ggregate Type	
	Natural Sand B			Traprock #13		
	Initial Final			Initial	Final	
	Reading	Reading	MBV	Reading	Reading	MBV
Test 1	13.0	24.0	5.5	22.5	37.0	7.3
Test 2	24.0	35.0	5.5	13.0	26.5	6.8
Test 3	2.0	13.0	5.5	26.5	40.0	6.8
	Δ	Vorage MPV -	55	1	Worago MRV -	60

Average MBV = 5.5 Average MBV = 6.9

C.9 SAND EQUIVALENT (AASHTO T104)

Table C.81 Fine Aggregate

	Aggregate Type Aggregate Type					Agg	regate Type		
	Natı	aral Sand A		Crushe	ed Gravel Sand		Natı	ural Sand B	-
	Clay Reading	Sand Reading	SE	Clay Reading	Sand Reading	SE	Clay Reading	Sand Reading	SE
Test 1	4.3	4.1	96.0	4.6	4.2	92.0	5.6	4.4	79.0
Test 2	4.2	4.1	98.0	4.6	4.0	87.0	5.5	4.3	79.0
Test 3	4.0	3.9	98.0	4.7	4.2	90.0	4.5	3.9	87.0
		Average SE =	98.0		Average SE =	90.0		Average SE =	82.0
	Agg	regate Type		Agg	gregate Type		Agg	regate Type	
	Gra	anite Sand	1	Do	lomite Sand		Dole	omite Sand	1
	Clay Reading	Sand Reading	SE	Clay Reading	Sand Reading	SE	Clay Reading	Sand Reading	SE
Test 1	4.3	3.3	77.0	4.0	4.0	100.0	4.5	3.5	78.0
Test 2	5.2	3.3	64.0	3.7	3.7	100.0	4.5	3.6	80.0
Test 3	5.3	3.5	67.0	3.9	3.9	100.0	4.6	3.6	79.0
		Average SE =	70.0		Average SE =	100.0		Average SE =	79.0
	Agg	regate Type		Agg	gregate Type				
	Tra	prock #16		Tr	aprock #13				
	Clay Reading	Sand Reading	SE	Clay Reading	Sand Reading	SE			
Test 1	3.9	3.4	88.0	4.5	3.7	83.0			
Test 2	4.5	3.7	83.0	5.9	4.0	68.0			
Test 3	-	-	-	6.9	4.0	58.0			
		Average SE =	86.0		Average SE =	70.0			

C.10 MAGNESIUM SULFATE SOUNDNESS TESTS (AASHTO T104) – 5 CYCLES

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
9.5mm to 4.75mm	0.0			0.0	0.0
4.75mm to 2.36mm	10.1	100.2	89.9	10.3	1.0
2.36mm to 1.18mm	30.7	100.2	88.4	11.8	3.6
1.18mm to .600mm	28.8	100.0	90.2	9.8	2.8
.600mm to .300mm	21.4	100.0	92.5	7.5	1.6
.300mm to .150mm	7.4				
Minus 150mm	1.6				
T 1	100.0				

Table C.82 Natural Sand A

Total = 100.0

Total = 9

Table C.83 Crushed Gravel Sand

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
9.5mm to 4.75mm	0.0			0.0	0.0
4.75mm to 2.36mm	18.2	100.4	89.7	10.7	1.9
2.36mm to 1.18mm	31.2	100.1	85.4	14.7	4.6
1.18mm to .600mm	19.8	100.3	80.9	19.3	3.8
.600mm to .300mm	13.6	100.7	82.6	18.0	2.4
.300mm to .150mm	9.9				
Minus 150mm	7.3				
Total =	100.0	•		Total =	13

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
9.5mm to 4.75mm	0.0			25.8	0.0
4.75mm to 2.36mm	14.7	100.0	74.2	25.8	3.8
2.36mm to 1.18mm	23.4	100.0	71.8	28.2	6.6
1.18mm to .600mm	24.4	100.0	67.7	32.3	7.9
.600mm to .300mm	19.5	100.0	68.7	31.3	6.1
.300mm to .150mm	11.9				
Minus 150mm	6.1				
Total =	100.0			Total =	24

Table C.84 Natural Sand B

Table C.85 Granite Sand

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
9.5mm to 4.75mm	1.0			12.9	0.1
4.75mm to 2.36mm	16.0	100.3	87.4	12.9	2.1
2.36mm to 1.18mm	26.0	100.4	80.7	19.6	5.1
1.18mm to .600mm	17.0	100.4	79.6	20.7	3.5
.600mm to .300mm	13.0	100.3	83.3	16.9	2.2
.300mm to .150mm	8.0				
Minus 150mm	19.0				
Total =	100.0			Total =	13

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
9.5mm to 4.75mm	1.6			12.1	0.2
4.75mm to 2.36mm	26.8	100	87.9	12.1	3.2
2.36mm to 1.18mm	39.9	100.0	92	8	3.2
1.18mm to .600mm	16.7	100.0	90.5	9.5	1.6
.600mm to .300mm	9.0	100.0	87.7	12.3	1.1
.300mm to .150mm	4.3				
Minus 150mm	1.7				
Total =	100.0			Total =	9

Table C.86 Dolomite Sand

Table C.87 Dolomite Sand

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
9.5mm to 4.75mm	0.0			0	0
4.75mm to 2.36mm	18.7	100.1	56.3	43.8	8.2
2.36mm to 1.18mm	29.6	100.0	66.4	33.6	9.9
1.18mm to .600mm	18.3	100.0	61.1	38.9	7.1
.600mm to .300mm	13.9	100.0	67.6	32.4	4.5
.300mm to .150mm	9.0				
Minus 150mm	10.5				
Total =	100.0			Total =	30

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
9.5mm to 4.75mm	5.0			3.4	0.2
4.75mm to 2.36mm	38.0	100.0	96.6	3.4	1.3
2.36mm to 1.18mm	23.0	100.0	90.9	9.1	2.1
1.18mm to .600mm	14.0	100.0	87.1	12.9	1.8
.600mm to .300mm	8.0	100.0	84.7	15.3	1.2
.300mm to .150mm	6.0				
Minus 150mm	6.0				
Total =	100.0			Total =	7

Table C.88 Traprock #16 Sand

Table C.89 Traprock #13 Sand

Sieve Size	Grading of Original Fractions Sample	Mass of Test Fraction Before Test	Mass of Test Fraction After Test	Percentage Passing Designated Sieve After Test	Weighted Percentage Loss
9.5mm to 4.75mm	4.0			6.8	0.3
4.75mm to 2.36mm	25.9	100.4	93.6	6.8	1.8
2.36mm to 1.18mm	21.0	100.2	78.8	21.4	4.5
1.18mm to .600mm	13.5	100.3	72.8	27.4	3.7
.600mm to .300mm	9.9	100.5	72.7	27.7	2.7
.300mm to .150mm	8.9				
Minus 150mm	16.8				
Total =	100.0			Total =	13

APPENDIX D

TEST SECTION CONSTRUCTION AND CONTROL

D.1 TEST SECTION CONSTRUCTION

Accelerated pavement tests were conducted at the INDOT/Purdue University APT facility located at the Indiana Department of Transportation Research Division in West Lafayette, Indiana (Figure D.1). In this facility, up to four test lanes can be constructed using conventional paving equipment. Prior to test section construction, any previously tested mixtures are removed. To facilitate removal, the slab heating system is turned on for 24 hours. A backhoe is used to remove the majority of the mixture with any remaining material being removed with hand tools.

Mixtures were produced by a local HMA contractor and delivered by truck to the APT facility. Mixtures were loaded into the paving machine hopper (Figure D.2) that is then backed into the facility (Figure D.3) Paving proceeds as the machine is driven forward, out of the building (Figure D.4). The resulting lane is 3 meters wide, half the total width of the APT test pit. The mixture is then compacted with a static steel-wheeled roller to achieve the target density (Figure D.5). A second mixture is placed in the adjacent 3-meter wide lane and compacted.

A longitudinal cut is made in the center of each 3-meter wide lane and the interior 1.5-meter wide portion of each lane removed (Figure D.6). The result is two, 1.5-meter wide test lanes, one on each side of the pit. Finally, a third mixture, 3-meters wide, is placed in the middle and compacted. The process produces four, 1.5-meter test lanes with three different mixtures; the middle two test lanes are constructed of the same mixture with no longitudinal joint between

them. Figure D.7 shows the APT test lanes and the APT equipment ready for testing. The average mat thickness was 100 mm.

It takes approximately seven weeks to construct, apply accelerated load passes, collect performance data, and perform material tests for each group of three test sections. The fourth section is a duplicate and is normally not tested unless additional data is needed. Actual construction, including the removal of any previously used mixtures, takes one day. Twenty-two (11 rutting, 5 moisture susceptibility, and 6 fatigue) test lanes were constructed and tested for the project.

D.2 SAMPLING AND TESTING DURING CONSTRUCTION

During construction of each set of test lanes, samples were taken and tested to determine: 1) The effect of HMA production and construction on aggregate properties; 2) in-place mixture composition; and 3) mixture volumetric properties. Prior to the HMA mixture production, bulk samples of aggregates were collected from plant stockpiles. Plant produced HMA samples were taken during construction and used to determine binder content, theoretical maximum specific gravity, and mixture gradations. In addition, samples were compacted with the gyratory compactor to N_{des} . Tests are summarized in Table D.1.

The aggregate mixture gradations for the coarse-graded, fine-graded, and moisture susceptibility mixtures are shown in Tables D.2, D.3, and D.4, respectively. The plant mixture properties of compacted HMA mixtures are shown in Table D.5. The data show that all of the mixtures had binder contents close to their design values. Gradations are also in close agreement except fines of plant produced material are lower. This value was slightly low for both the plant

mixtures and for cores taken before traffic. The VTM values of these mixtures were high. Also, these mixtures did not meet VFA requirements because of high VTM values.

Once construction was completed cores were extracted from each lane and tested for in-place density and moisture sensitivity (for the stripping portion of the study). In addition, after binder extraction, mixture gradation, fine aggregate uncompacted voids content, and aggregate particle flat *or* elongated percentage were determined. Figure D.8 shows the approximate location of cores extracted from test lanes. Cores were extracted before and after traffic. Three cores, one each from sections four, five, and six of each test lane, were extracted prior to APT trafficking. After APT trafficking four cores were extracted from each of the three sections, i.e. section four, five, and six (12 total). In each of the three sections, two cores were extracted from outside the wheel path and two from within the wheel path. Core diameters were approximately 102 mm (4 inches). Figure D.9 shows twelve cores taken after traffic.

Sample Type	Test Method	Experi- ments	Use
Aggregate Samples	Aggregate Gradation (AASHTO T11 and T27)	Rutting Moisture Fatigue	Composition Performance Relationships
	Binder Content (AASHTO T164)	Rutting Moisture Fatigue	Composition Performance Relationships Volumetric Properties
Aggregate Gradation (AASHTO T11 and T27)		Rutting Moisture Fatigue	Composition Performance Relationships
Loose Mixture	Theoretical Maximum Specific Gravity (AASHTO T209)	Rutting Moisture Fatigue	Composition Performance Relationships Volumetric Properties
	Flat <i>or</i> Elongated Particles in Coarse Aggregate, 2:1 Ratio (ASTM D4791)	Rutting	Performance Relationships
	Uncompacted Voids Content of Fine Aggregate (ASTM C1252)	Rutting Moisture	Performance Relationships
	Bulk Specific Gravity of Compacted Bituminous Mixture (AASHTO T166)	Rutting Moisture	Composition Performance Relationships Volumetric Properties
Cores	Moisture Sensitivity (AASHTO T283)	Moisture	Volumetric Properties Changes in Strength
	Flat <i>or</i> Elongated Particles in Coarse Aggregate, 2:1 Ratio (ASTM D4791)		Performance Relationships
	Uncompacted Voids Content of Fine Aggregate (ASTM C1252)	Rutting Moisture	Performance Relationships

Table D.1 APT Material Sampling and Testing Plan

	CA-1		CA	CA-2		A-3
Sieve Size, mm	Design	Plant	Design	Plant	Design	Plant
12.5	100	100.0	100.0	100.0	100.0	100.0
9.5	91.0	89.2	91.5	91.7	89.0	97.8
4.75	48.3	48.5	47.3	51.3	43.5	45.5
2.36	29.3	30.8	30.8	33.6	29.1	30.8
1.18	20.7	20.3	21.2	21.8	20.5	20.7
0.60	13.2	12.0	13.8	12.8	13.0	11.7
0.30	7.6	5.2	8.3	5.7	7.5	4.3
0.15	4.9	2.8	5.6	3.2	4.8	2.3
0.075	3.2	2.1	4.0	2.5	3.2	1.7
Pb, %	5.7	5.6	6.1	5.9	3.9	3.8

Table D.2 Plant Produced HMA Mixture Gradation, Coarse-Graded Mixtures

Table D.2 Continued

	CA	A -4	CA-5			
Sieve Size, mm	Design	Plant	Design	Plant		
12.5	100.0	100.0	100.0	100.0		
9.5	93.1	91.5	89.8	89.7		
4.75	46.9	44.8	37.8	40.0		
2.36	31.1	25.4	29.1	28.4		
1.18	22.5	15.6	21.7	19.0		
0.60	14.3	8.9	14.2	10.4		
0.30	8.0	4.2	6.9	4.6		
0.15	5.4	2.5	3.7	2.7		
0.075	3.4	2.0	2.5	2.0		
Pb, %	5.8	5.4	4.8	4.6		

	FA-1		FA	A- 2	FA-3		
Sieve Size, mm	Design	Plant	Design	Plant	Design	Plant	
12.5	100.0	100.0	100.0	100.0	100.0	100.0	
9.5	93.6	98.2	94.1	98.1	94.4	98.3	
4.75	67.2	64.8	69.7	66.1	71.3	68.1	
2.36	54.7	53.9	52.5	46.5	56.3	49.7	
1.18	36.8	36.5	33.4	27.7	41.4	32.4	
0.60	20.4	19.8	21.6	16.7	26.3	17.3	
0.30	8.2	6.4	13.4	9.9	14.2	8.6	
0.15	3.6	2.4	7.0	5.3	6.4	3.9	
0.075	2.2	1.7	3.9	2.9	3.8	2.4	
Pb, %	6.0	6.3	5.7	5.6	5.8	5.4	
Film Thickness, µm	12.75	14.37	10.33	12.65	9.04	11.88	

Table D.3 Plant Produced HMA Mixture Gradations, Fine-Graded Mixtures

Table D.3 Continued

	FA	- 4	FA	-5	FA-6		
Sieve Size, mm	Design	Plant	Design	Plant	Design	Plant	
12.5	100.0	100.0	100.0	100.0	100.0	100.0	
9.5	94.4	97.2	93.6	98.5	98.3	99.6	
4.75	70.7	70.9	67.2	64.7	70.2	76.0	
2.36	54.3	51.9	49.2	48.3	49.3	49.8	
1.18	37.1	33.6	31.1	31.0	34.5	32.5	
0.60	26.1	23.0	20.1	18.5	25.1	22.3	
0.30	17.6	16.3	11.8	10.7	18.1	15.7	
0.15	12.4	10.7	6.4	6.1	11.9	10.9	
0.075	8.5	6.4	3.5	4.1	6.9	7.6	
Pb, %	4.9	5.2	6.3	6.1	4.9	5.2	
Film Thickness, µm	5.86	7.38	11.02	11.44	6.09	6.52	

	FAM1		FA	M2	FAM3		
Sieve Size, mm	Design	Plant	Design	Plant	Design	Plant	
12.5	100.0	99.6	100.0	100.0	100.0	100.0	
9.5	94.8	93.8	95.5	95.0	95.5	94.8	
4.75	70.0	63.6	73.7	67.3	73.1	70.8	
2.36	55.0	51.8	54.5	46.8	54.4	50.8	
1.18	37.7	35.4	35.5	27.3	37.2	32.2	
0.60	21.9	20.1	23.6	16.3	26.1	21.5	
0.30	10.1	7.6	15.4	10.0	17.7	15.2	
0.15	5.3	3.8	8.7	5.9	12.5	10.8	
0.075	3.4	3.0	5.0	3.9	8.5	7.6	
Pb, %	6.1	5.7	6.35	6.25	5.4	5.4	
Film	10.91	11.67	9.46	12.32	6.37	7.21	
Thickness, µm							

Table D.4 Plant Produced HMA Mixture Gradations, Moisture Susceptibility Mixtures

Table D.4 Continued

	FA	M4	FAM5			
Sieve Size, mm	Design	Plant	Design	Plant		
12.5	100.0	99.7	100.0	100.0		
9.5	95.5	96.4	95.2	95.1		
4.75	71.2	71.7	72.3	74.6		
2.36	46.1	45.0	54.7	58.2		
1.18	32.1	29.4	40.3	41.3		
0.60	23.3	20.6	25.6	25.0		
0.30	16.8	15.3	13.9	14.3		
0.15	11.0	11.0	6.3	7.0		
0.075	6.4	7.5	3.7	4.4		
Pb, %	5.3	5.5	6.1	5.9		
Film	6.81	7.13	10.01	9.03		
Thickness, µm						

Coarse-Graded Mixtures											
Mixture ID	ł	P _b	VTN	1	G _{mm}	VM	VMA, %		VFA, %		%G _{mm}
	ç	%	%			N _{des}	Req'd.	N _{des}	Req	d. %	N _{ini}
CA-1	5	.6	5.2		2.520	16.3		68.4	Ļ	0.6	86.4
CA-2	6	.0	4.1		2.443	14.2	14.0	70.9	65	0.7	87.0
CA-3	3	.8	7.1		2.507	14.0	min.	49.1	to	0.5	86.8
CA-4	5	.4	9.8		2.457	20.8	111111.	52.9) 75	0.4	82.1
CA-5	4	.6	7.1		2.643	16.4		57.0)	0.5	85.2
	Fine-Graded Mixtures										
Mixture ID	ł	b	VTN	1	G _{mm}	VM			/FA, %	DP	%G _{mm}
	ç	%	%			N _{des}	Req'd.	N _{des}	Req	d. %	N _{ini}
FA-1	6	.3	4.9		2.449	15.9		68.9)	0.4	90.1
FA-2	5	.6	6.8		2.454	18.2		62.6	65	0.6	85.5
FA-3	5	.4	5.5		2.454	16.0	14.0	65.4	_	0.6	87.0
FA-4	5	.2	3.6		2.449	14.8	min.	75.4	to 15	1.3	88.2
FA-5	6	.1	2.9		2.457	14.5		80.1	15	0.9	88.3
FA-6	5	.2	3.6		2.613	14.7		75.2	2	1.7	87.6
					Moistur	e Suscep	tibility N	lixture	s		
Mixture	e	Pb	V	ΓМ	G	VM	A, %	VF	A, %	DP	%G _{mm}
ID		%	(%	G _{mm}	N _{des}	Req'd	N _{des}	Req'd.	%	N _{ini}
FAM1		5.7	' 4	.1	2.483			72.6		0.6	90.2
FAM2		6.2	5 5	.3	2.486	17.8		70.2		0.7	85.6
FAM3		5.4	- 3	.5	2.487	15.0	14.0	76.4	65-75	1.5	86.7
FAM4		5.5	5 3	.2	2.633	15.1		78.7		1.6	86.2
FAM5		5.9) 3	.1	2.472	14.8		79.0		0.9	87.1

Table D.5 Plant Produced HMA Mixture Volumetric Data



Figure D.1 INDOT/ Purdue Accelerated Pavement Testing Facility



Figure D.2 Loading the paver



Figure D.3 The paving machine backs into the facility



Figure D.4 Test section paving



Figure D.5 Mixture compaction



Figure D.6 Center section is removed



Figure D.7 Test sections are ready for testing

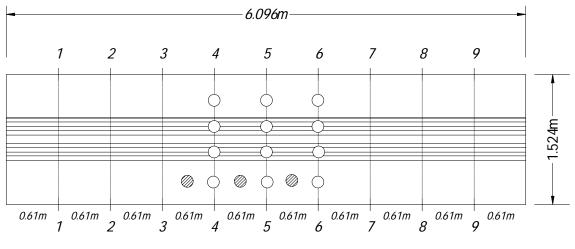


Figure D.8 APT coring locations



Figure D.9 Cores taken after traffic

APPENDIX E

MOISTURE SUSCEPTIBILITY

E.1 DRY AND CONDITIONS SPECIMEN PHOTOGRAPHS

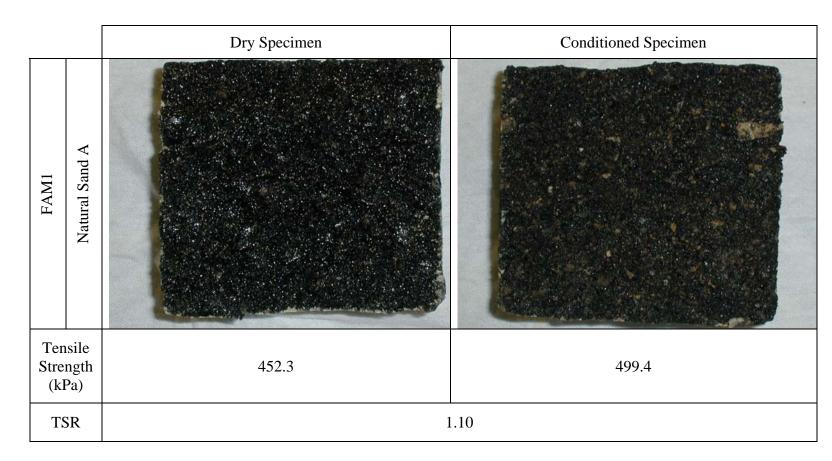


Figure E.1 Dry and conditioned specimens of FAM1 mixture

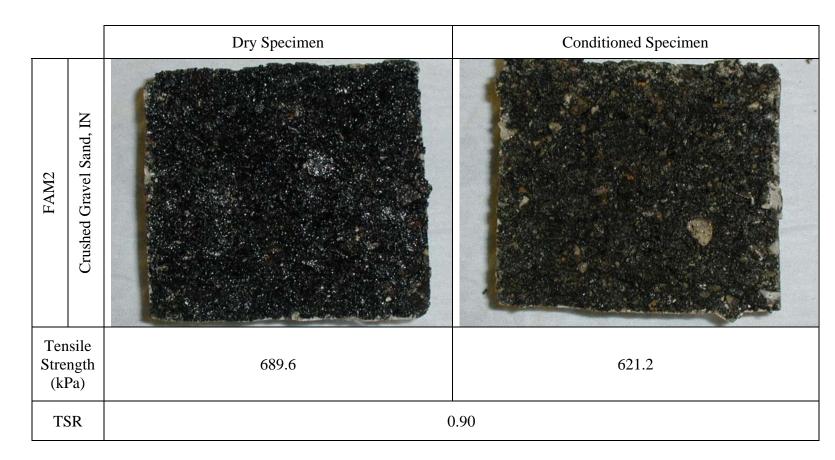


Figure E.2 Dry and conditioned specimens of FAM2 mixture

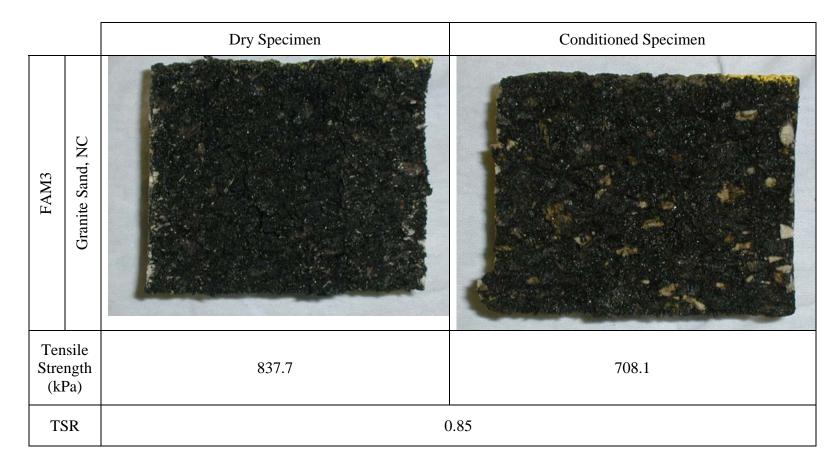


Figure E.3 Dry and conditioned specimens of FAM3 mixture

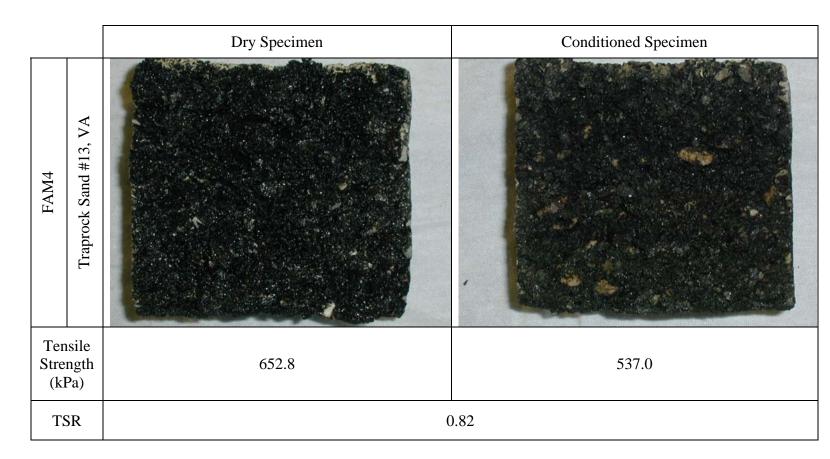


Figure E.4 Dry and conditioned specimens of FAM4 mixture

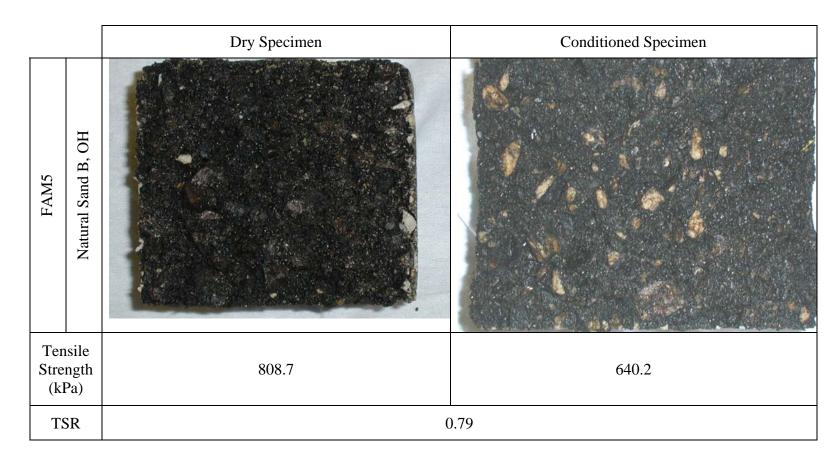


Figure E.5 Dry and conditioned specimens of FAM5 mixture



Figure E.6 Cores from FAM1 test section after traffic

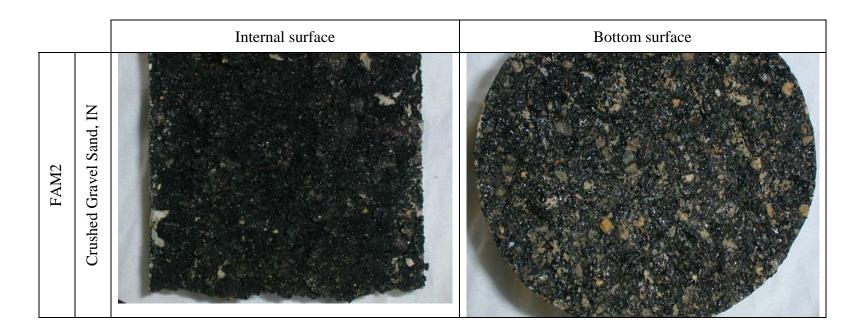


Figure E.7 Cores from FAM2 test section after traffic



Figure E.8 Cores from FAM3 test section after traffic



Figure E.9 Cores from FAM4 test section after traffic



Figure E.10 Cores from FAM5 test section after traffic

APPENDIX F

BIBLIOGRAPHY

Alhozaimy, A. M., "A Correlation Between Material Finer than No. 200 Sieve and Sand Equivalent Tests for Natural and Crushed Stone Sands," Cement, Concrete, and Aggregates, CCAGDP, Vol. 20, December 1998, pp. 221-226

Buchanan, M. S., "Evaluation of the Effect of Flat and Elongated Particles on the Performance of Hot Mix Asphalt Mixtures," NCAT Report No. 2000-03, May 2000

Cominsky, R., Leahy, R. B., Harrigan, E. D., Level One Mix Design: Material Selection, Compaction, and Conditioning, Strategic Highway Research Program, SHRP-A-408, National Research Council, Washington, DC, 1994

Hudson, B. P., "Potential Errors in the FAA Test," A paper presented at the 8th Annual Symposium of the International Center for Aggregates Research, Denver, Colorado, April 2000.

Ishai, I. and Craus, J., "Effects of Some Aggregate and Filler Characteristics on Behavior and Durability of Asphalt Paving Mixtures," Transportation Research Record 1530, Transportation Research Board, Washington, D.C., 1997, pp. 75-85.

Kandhal, P. S., Foo, K. Y., and Mallick, R. B., "Critical Review of Voids in Mineral Aggregate Requirements in Superpave," Transportation Research Record 1609, Transportation Research Board, Washington, D.C., 1998, pp. 21-27.

Kennedy, T. W., Huber, G. A., Harrigan, E. T., Cominsky, R. J., Hughes, C. S., Von Quintus, H. L., and Moulthrop, J. S., "Superior Performing Asphalt Pavements (Superpave): The Product of the SHRP Asphalt Research Program," Strategic Highway Research Program, SHRP-A-410, 1994

Mix Design Methods for Asphalt Concrete (MS-2), Asphalt Institute, Lexington, Kentucky

Rismantojo, E., Haddock, J. E., and White, T. D., "Comparison of Fine Aggregate Uncompacted Voids Measured by the ASTM C1252 and VTM-5 Devices." Journal of Testing and Evaluation, ASTM International, Vol. 31, No. 4, July 2003.

Rogers, C. A., Bailey, M. L., and Price, B, "Micro-Deval Test for Evaluating the Quality of Fine Aggregate for Concrete and Asphalt," Transportation Research Record 1301, Transportation Research Board, Washington, D. C., 1991, pp. 68-76.

Senior, S. A. and Rogers, C. A., "Laboratory Tests for Predicting Coarse Aggregate Performance in Ontario," Transportation Research Record 1301, Transportation Research Board, Washington, D. C., 1991, pp. 97-106.

Tourenq, C. and Tran, N. L., "Assessment of Clays in Aggregates. A Track for Further Research," Bulletin of the International Association of Engineering Geology, No. 56, October 1997, pp. 97-102.

Vadakpat, G. and Stonex, A., "Impacts of Implementation of Superpave Aggregate Consensus Property Tests in Pennsylvania," A paper presented at the 79th Annual Meeting of the Transportation Research Board, Washington, D.C., January 2000.

West, T. R., "Geology Applied to Engineering," Prentice Hall, Englewood Cliffs, New Jersey, 1995

White, T.D., Zaghloul, S. M., Anderton, G. L., Smith, D. M., "Pavement Analysis for Moving Aircraft Load," Journal of Transportation Engineering, November/December, 1997, pp. 436-446